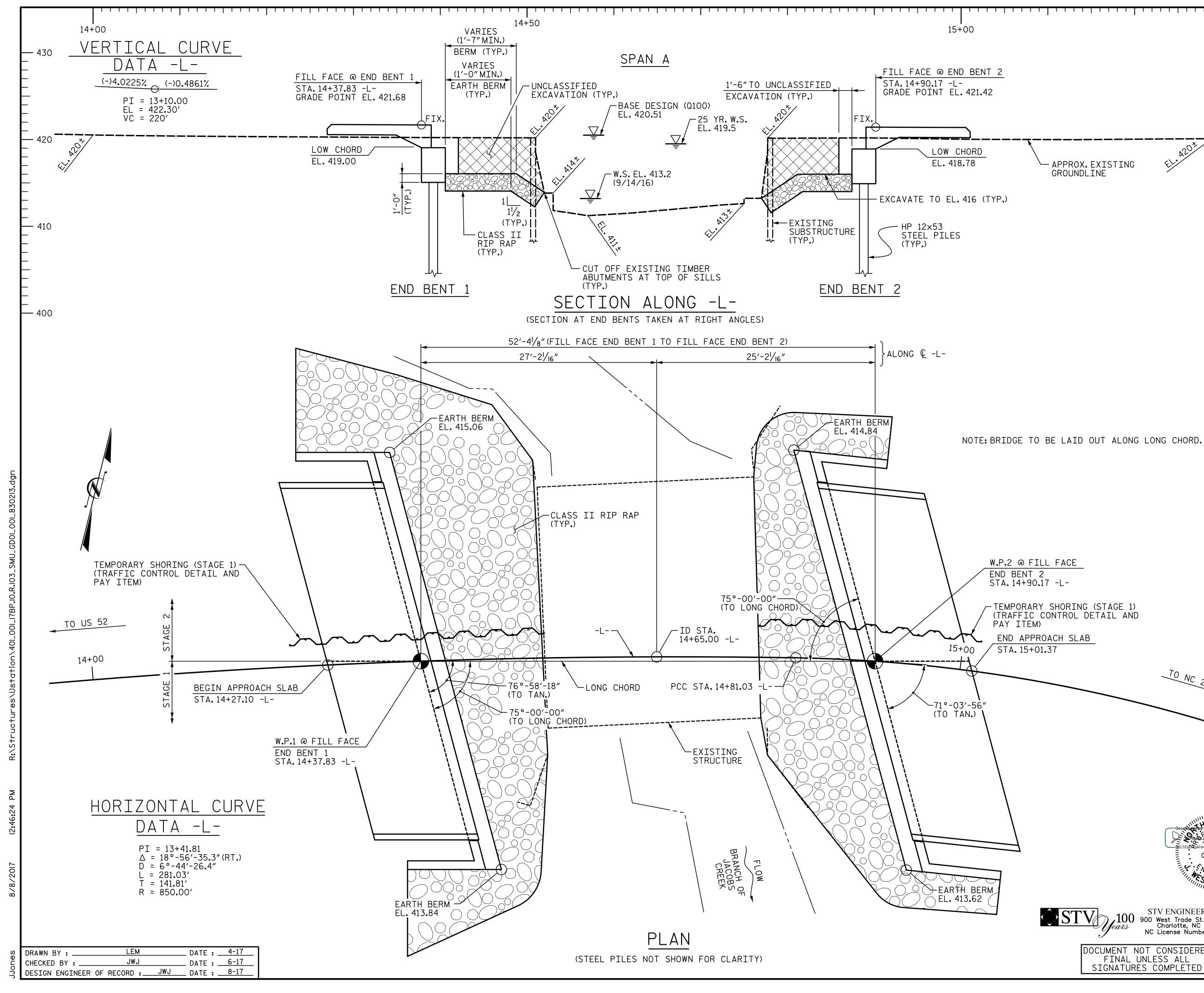
+

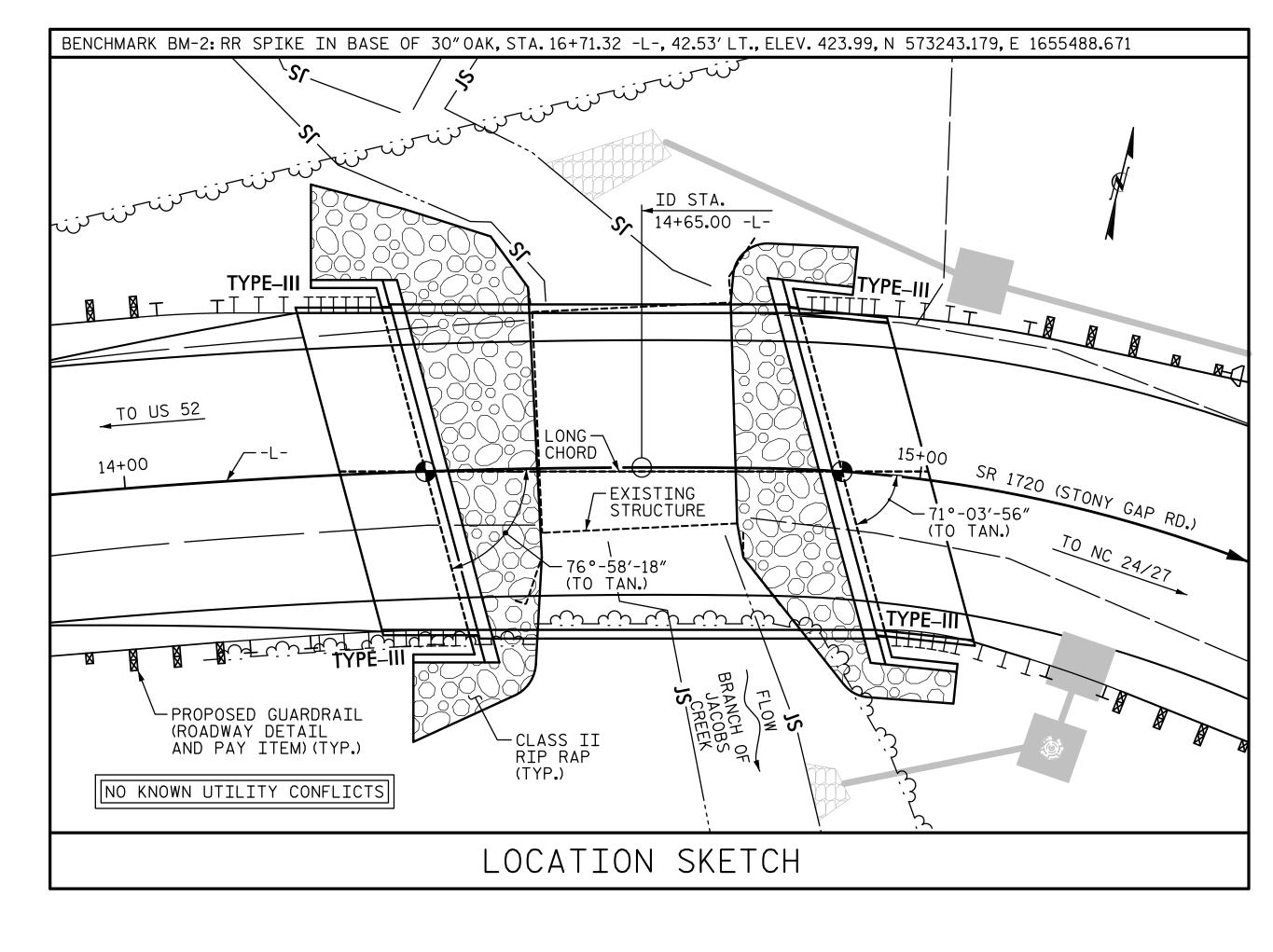


11/

HORIZONTAL CURVE DATA -L-PI = 15+93.30 ∆ = 65°-21′-56.9″(RT.) D = 32°-44′-25.6″ L = 199.65' T = 112.27' R = 175.00'

TO NC 24/27	
	PROJECT NO. <u>17BP.10.R.103</u> STANLY COUNTY
	STATION: 14+65.00 -L-
Varian STV ENGINEERS, INC. 900 West Trade St., Suite 715 Charlotte, NC 28202 NC License Number F-0991	SHEET 1 OF 2 STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH GENERAL DRAWING FOR BRIDGE ON SR 1720 (STONY GAP RD.) OVER BRANCH OF JACOBS CREEK BETWEEN US 52 AND NC 24/27
UMENT NOT CONSIDERED FINAL UNLESS ALL IGNATURES COMPLETED	REVISIONS SHEET NO. NO. BY: DATE: NO. BY: DATE: S-1 1 3 And Streets Streets Streets 18 2 4 1 18 18 18 18

—



HYDRAULIC DATA

DESIGN DISCHARGE:	900 CFS
FREQUENCY OF DESIGN FLOOD:	25 YRS.
DESIGN HIGH WATER ELEVATION:	419.5
DRAINAGE AREA:	2.45 SQ.MI.
BASE DISCHARGE (Q100):	1,300 CFS
BASE HIGH WATER ELEVATION:	420.51

OVERTOPPING DATA

OVERTOPPING DISCHARGE: ______1,600 CFS FREQUENCY OF OVERTOPPING: _____200+ YRS. OVERTOPPING FLOOD ELEVATION: _____ 421.8

	TOTAL BILL OF MATERIAL															
REMOVAL OF EXISTING STRUCTURE STRUCTURE EXCAVATION REINFORCING STRUCTURE EXCAVATION REINFORCING STEEL SLABS REINFORCING STEEL PILES PILE DRIVING EQUIPMENT SETUP FOR HP12X53 STEEL PILES						VERTICAL CONCRETE BARRIER RAIL	RIP RAP CLASS II (2'-0" THICK)	GEOTEXTILE FOR DRAINAGE	ELASTOMERIC BEARINGS	PRES	9″X 1'-9″ STRESSED NCRETE ED SLABS	ASBESTOS ASSESSMENT				
		LUMP SUM	LUMP SUM	CU.YD.	LUMP SUM	LBS.	NO.	LIN.FT.	EA.	LIN.FT.	TONS	SQ.YDS.	LUMP SUM	NO.	LIN.FT.	LUMP SUM
SUPER-	STAGE 1									50.0				7	350.0	
STRUCTURE	STAGE 2									50.0				7	350.0	
END BENT 1	STAGE 1			13.0		1,609	4	80	4							
ENU DENT 1	STAGE 2			13.0		1,546	3	45	3		120	130				
	STAGE 1			13.2		1,620	4	80	4							
END BENT 2	STAGE 2			12.9		1,549	3	75	3		95	105				
ТОТ	AL	LUMP SUM	LUMP SUM	52.1	LUMP SUM	6,324	14	280	14	100.0	215	235	LUMP SUM	14	700.0	LUMP SUM

S	DRAWN BY :	LEM	D	ATE :4-:	17
ЪС	CHECKED BY :	JMJ	D	ATE :6-:	17
oll	DRAWN BY : CHECKED BY : _ DESIGN ENGINE	ER OF RECORD	JWJ D	ATE : 8-1	17

0,46,26 PM R.\S+rinctince\listation\401 003 17BP 10 R 103 SM11 GD02 002 830213 dam

<u>GENERAL NOTES</u>

ASSUMED LIVE LOAD = HL-93 OR ALTERNATE LOADING.

THIS BRIDGE HAS BEEN DESIGNED IN ACCORDANCE WITH THE AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS. THIS BRIDGE IS LOCATED IN SEISMIC ZONE 1.

FOR OTHER DESIGN DATA AND GENERAL NOTES, SEE "STANDARD NOTES" SHEET.

FOR EROSION CONTROL MEASURES, SEE EROSION CONTROL PLANS.

THE EXISTING STRUCTURE CONSISTING OF (1)25'-7"± STEEL PLANK DECK ON STEEL I-BEAMS SPAN WITH A CLEAR ROADWAY WIDTH OF 27'-11" ON TIMBER CAPS, TIMBER PILES ENCASED IN CONCRETE, AND TIMBER BULKHEADS AND LOCATED AT THE PROPOSED STRUCTURE SHALL BE REMOVED IN STAGES. THE EXISTING BRIDGE IS PRESENTLY POSTED FOR LOAD LIMIT.

REMOVAL OF THE EXISTING BRIDGE SHALL BE PERFORMED SO AS NOT TO ALLOW DEBRIS TO FALL INTO THE WATER. THE CONTRACTOR SHALL REMOVE THE BRIDGE AND SUBMIT PLANS FOR DEMOLITION FOR REVIEW IN ACCORDANCE WITH ARTICLE 402-2 OF THE STANDARD SPECIFICATIONS.

INASMUCH AS THE PAINT SYSTEM ON THE EXISTING STRUCTURAL STEEL CONTAINS LEAD, THE CONTRACTOR'S ATTENTION IS DIRECTED TO ARTICLE 107-1 OF THE STANDARD SPECIFICATIONS. ANY COSTS RESULTING FROM COMPLIANCE WITH APPLICABLE STATE OR FEDERAL REGULATIONS PERTAINING TO HANDLING OF MATERIALS CONTAINING LEAD BASED PAINT SHALL BE INCLUDED IN THE BID PRICE FOR "REMOVAL OF EXISTING STRUCTURE".

THE MATERIAL SHOWN IN THE CROSS-HATCHED AREA (ON SHEET 1 OF 2) SHALL BE EXCAVATED FOR A DISTANCE FROM THE CENTERLINE OF ROADWAY OF APPROXIMATELY 34 FT± (LEFT) AND 29 FT± (RIGHT) AT END BENT 1 AND 29 FT± (LEFT AND RIGHT) AT END BENT 2 AS DIRECTED BY THE ENGINEER. THIS WORK WILL BE PAID FOR AT THE CONTRACT LUMP SUM PRICE FOR UNCLASSIFIED STRUCTURE EXCAVATION. SEE SECTION 412 OF THE STANDARD SPECIFICATIONS.

THE SUBSTRUCTURE OF THE EXISTING BRIDGE INDICATED ON THE PLANS IS FROM THE BEST INFORMATION AVAILABLE. SINCE THIS INFORMATION IS SHOWN FOR THE CONVENIENCE OF THE CONTRACTOR, THE CONTRACTOR SHALL HAVE NO CLAIM WHATSOEVER AGAINST THE DEPARTMENT OF TRANSPORTATION FOR ANY DELAYS OR ADDITIONAL COST INCURRED BASED ON DIFFERENCES BETWEEN THE EXISTING BRIDGE SUBSTRUCTURE SHOWN ON THE PLANS AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

THIS STRUCTURE HAS BEEN DESIGNED IN ACCORDANCE WITH "HEC 18 - EVALUATING SCOUR AT BRIDGES". FOR SUBMITTAL OF WORKING DRAWINGS, SEE SPECIAL PROVISIONS.

FOR FALSEWORK AND FORMWORK, SEE SPECIAL PROVISIONS.

FOR CRANE SAFETY, SEE SPECIAL PROVISIONS.

FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.

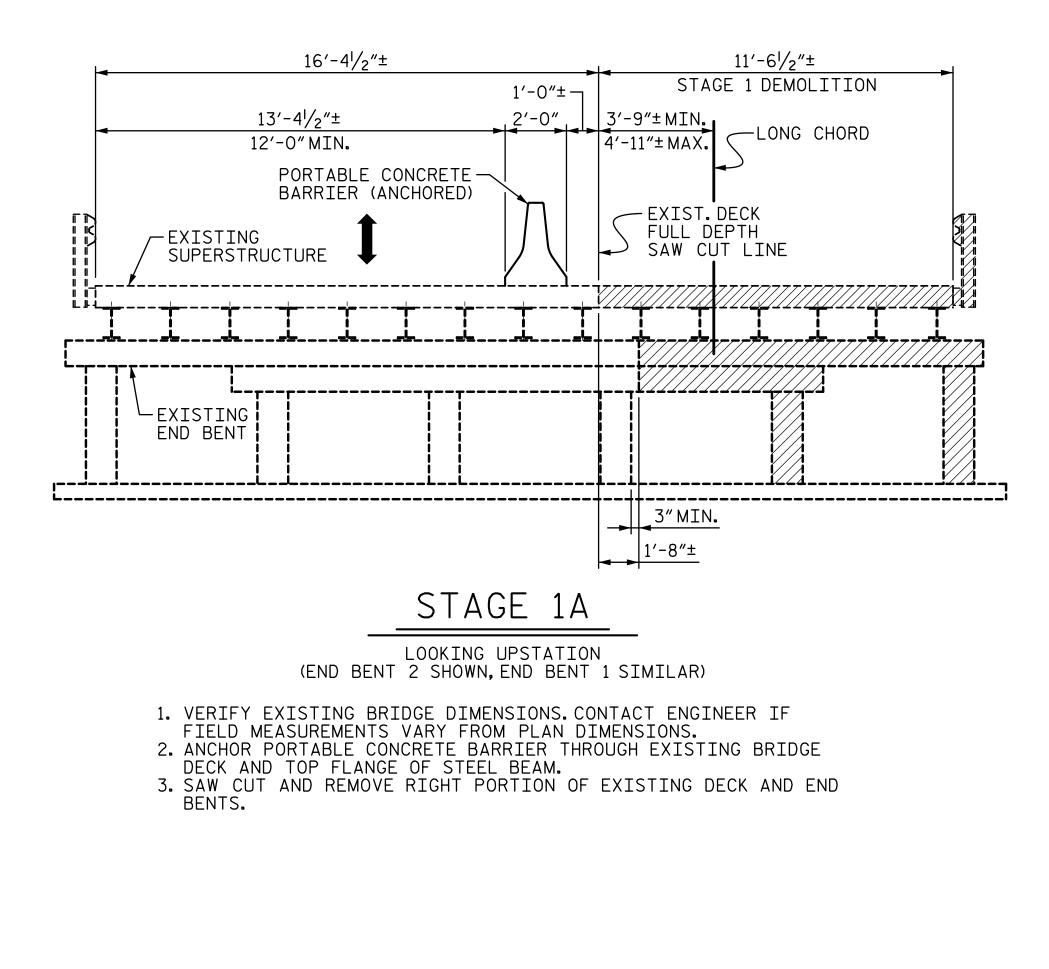
ASPHALT WEARING SURFACE IS INCLUDED IN ROADWAY QUANTITY ON ROADWAY PLANS. TEMPORARY SHORING WILL BE REQUIRED IN THE AREAS INDICATED IN THE PLAN VIEW.FOR TEMPORARY SHORING, SEE SPECIAL PROVISIONS. FOR ASBESTOS ASSESSMENT FOR BRIDGE DEMOLITION AND RENOVATION ACTIVITIES, SEE SPECIAL PROVISIONS.

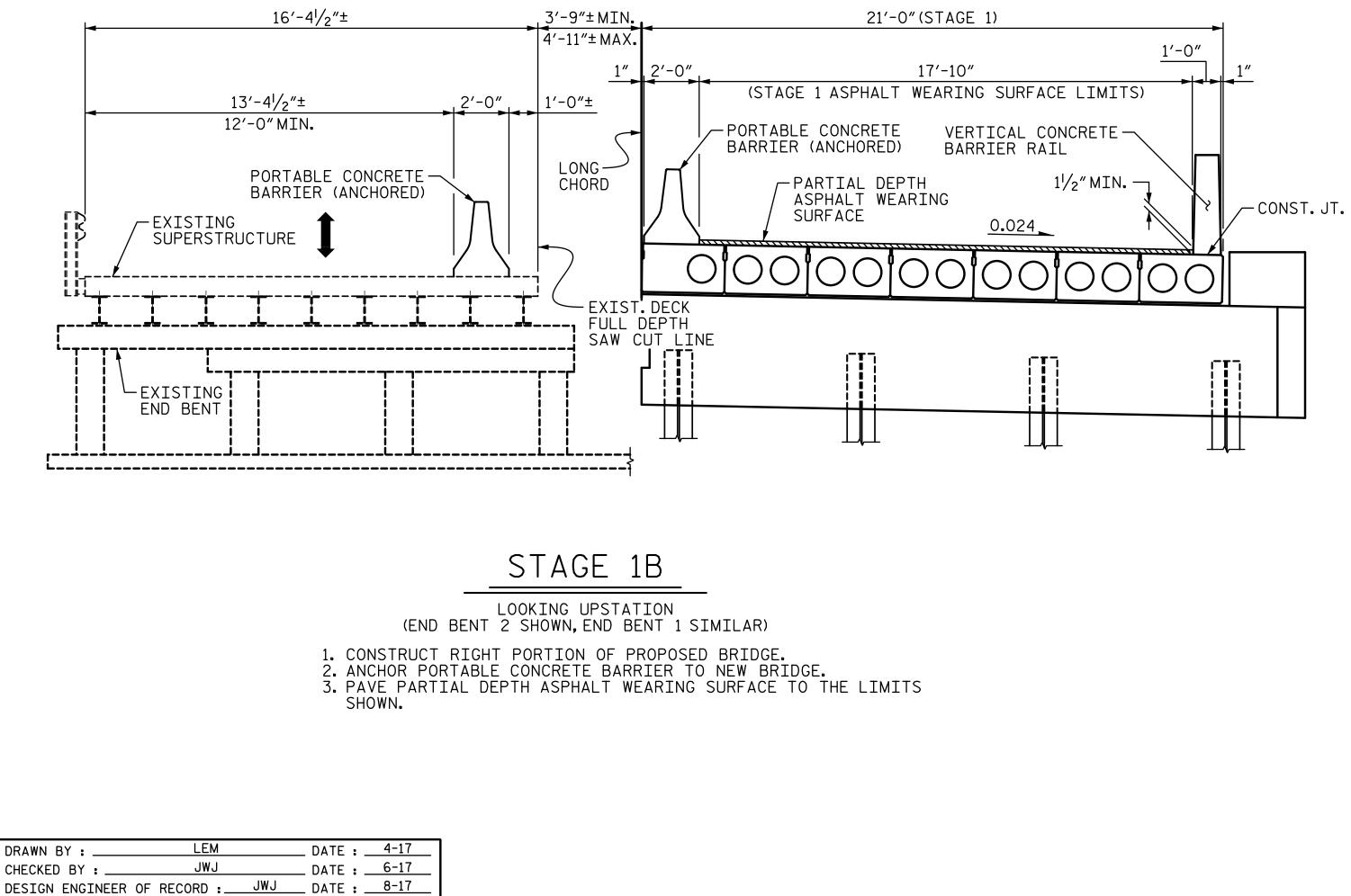
FOUNDATION NOTES

FOR PILES, SEE GEOTECHNICAL SPECIAL PROVISIONS AND SECTION 450 OF THE STANDARD SPECIFICATIONS. PILES AT END BENT 1 ARE DESIGNED FOR A FACTORED RESISTANCE OF 90 TONS PER PILE. DRIVE PILES AT END BENT 1 TO A REQUIRED DRIVING RESISTANCE OF 150 TONS PER PILE. PILES AT END BENT 2 ARE DESIGNED FOR A FACTORED RESISTANCE OF 90 TONS PER PILE. DRIVE PILES AT END BENT 2 TO A REQUIRED DRIVING RESISTANCE OF 150 TONS PER PILE. REMOVE (EXCAVATE) ALL RIP RAP AT END BENT 2 PRIOR TO DRIVING PILES.

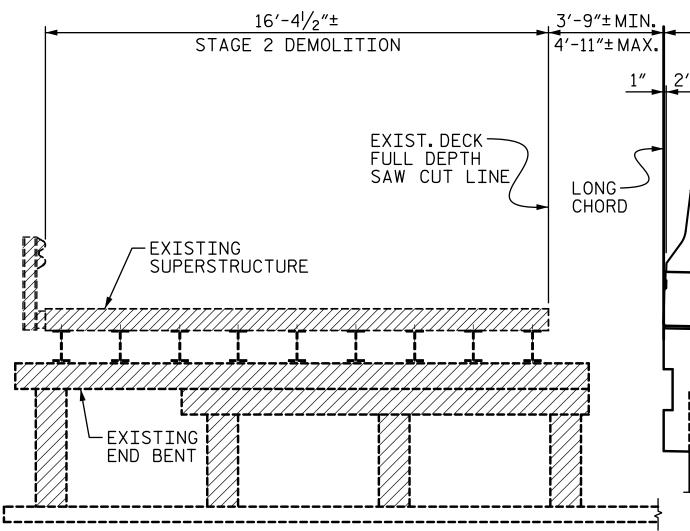


	PROJECT NO. 178P.10.R.103									
		OUNTY								
	STATI									
	SHEET 2 0	F 2								
Docustored by CARO	STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH									
038640	G	ENERA	AL D	RAWI	NG					
<i>VG</i> (NEL SLEY 8/8/2017	FOR BRIDGE ON SR 1720 (STONY GAP RD.) OVER BRANCH									
0 STV ENGINEERS, INC. 900 West Trade St., Suite 715 Charlotte, NC 28202 NC License Number F-0991	OF JACOBS CREEK BETWEEN US 52 AND NC 24/27									
NOT CONSIDERED	NO. BY:	REVIS DATE:	IONS	DATE:	SHEET NO. S-2					
UNLESS ALL JRES COMPLETED	1	DATE:	<u>अ</u>		TOTAL SHEETS 18					





+

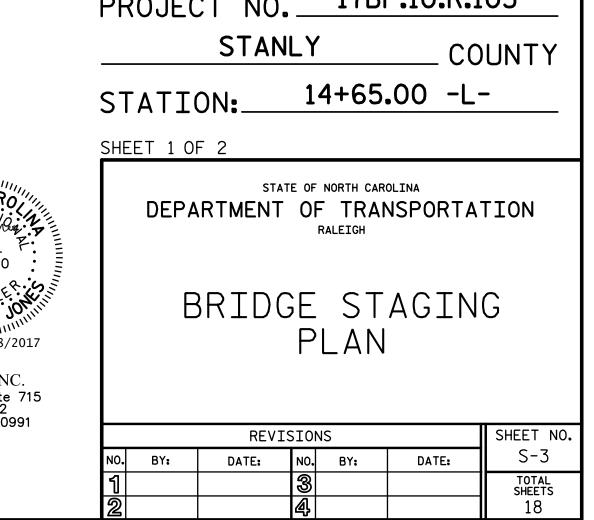


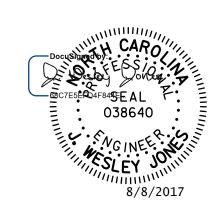
21'-0" (STAGE 1) 1'-0" 17'-10" 2'-0" - PORTABLE CONCRETE VERTICAL CONCRETE — BARRIER (ANCHORED) + BARRIER RAIL - PARTIAL DEPTH 1[|]∕₂″MIN.− ASPHALT WEARING ← CONST.JT. SURFACE 0.024 OOSTAGE 2A LOOKING UPSTATION (END BENT 2 SHOWN, END BENT 1 SIMILAR) SHIFT TRAFFIC TO NEWLY CONSTRUCTED BRIDGE.
 REMOVE REMAINDER OF EXISTING SUPERSTRUCTURE AND END BENTS. PROJECT NO. ____17BP.10.R.103 STANLY



NOTES:

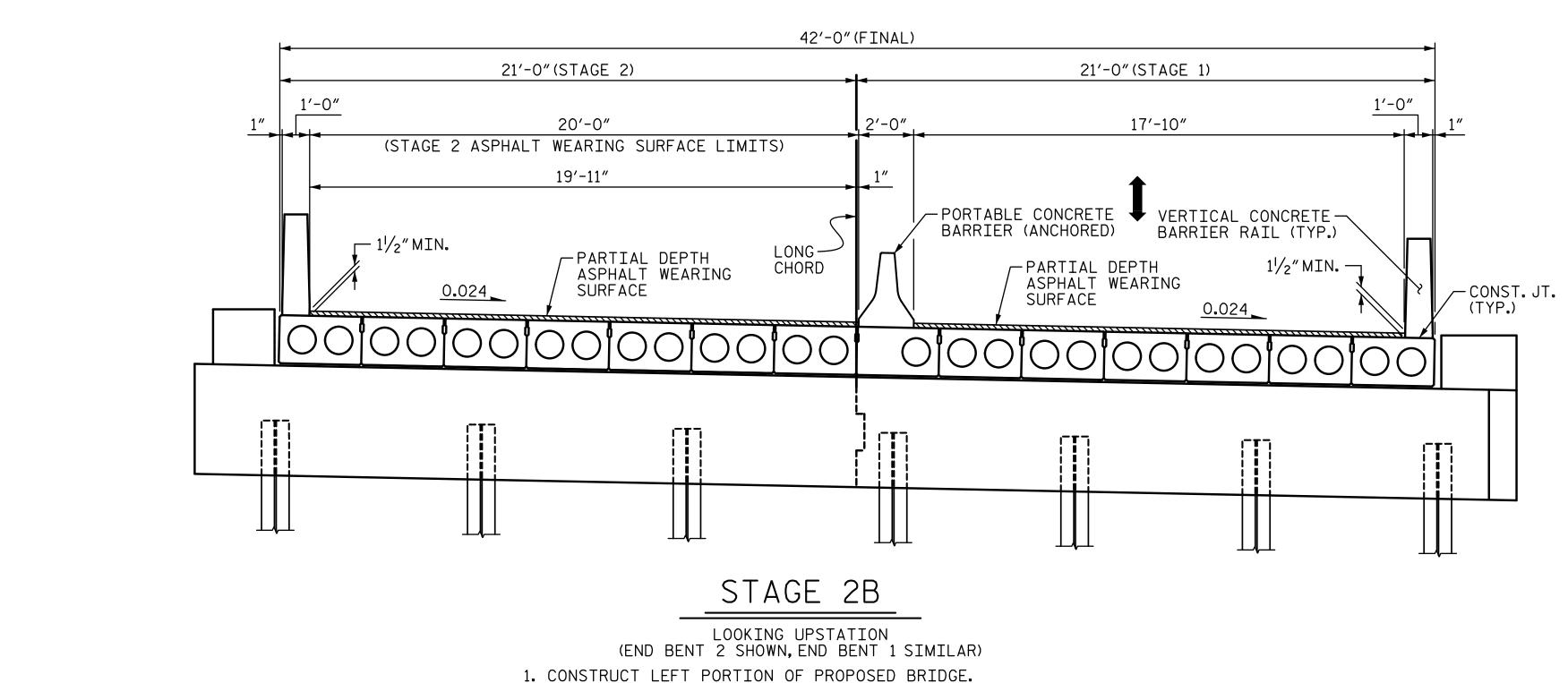
CONTRACTOR TO VERIFY LOCATION AND DIMENSIONS OF EXISTING BRIDGE. STAGED DEMOLITION SHALL BE DETERMINED BY CONTRACTOR AND APPROVED BY THE ENGINEER. DIMENSIONS ARE NORMAL TO 🕻 BRIDGE UNLESS OTHERWISE NOTED.

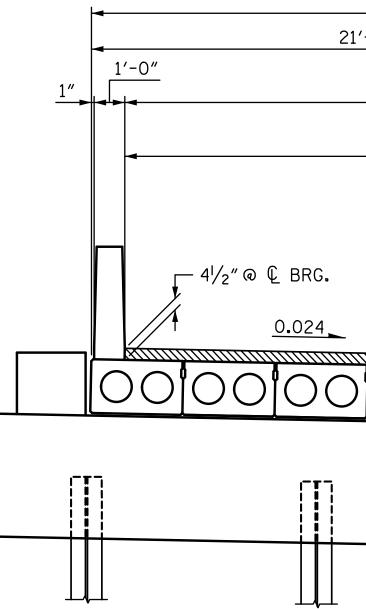




STV ENGINEERS, INC. 900 West Trade St., Suite 715 Charlotte, NC 28202 NC License Number F-0991

JMENT	NOT	CONSIDERED
		ESS ALL
GNATU	RES	COMPLETED





8/8/2017					
S	DRAWN BY :	LEM		DATE : _	4-17
ЧС	CHECKED BY :	JWJ		DATE :_	6-17
JJones	DESIGN ENGINEER	OF RECORD :	JMN	DATE :_	8-17

2. PAVE PARTIAL DEPTH ASPHALT WEARING SURFACE TO THE LIMITS SHOWN.

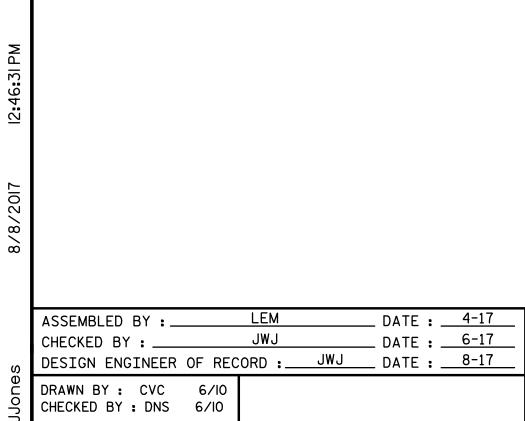
42'-0"(FINAL) 21'-0"(STAGE 2) 21'-0"(STAGE 1) 1'-0" 39′-10″ (STAGE 3 ASPHALT WEARING SURFACE LIMITS) 19'-11″ 19'-11″ VERTICAL CONCRETE — BARRIER RAIL (TYP.) LONG -----CHORD FULL DEPTH 4[|]∕2″@ € BRG. SURFACE ← CONST. JT. (TYP.) 0.024 00|00|00|00 0001001001001001001 STAGE 3 LOOKING UPSTATION (END BENT 2 SHOWN,END BENT 1 SIMILAR)

REMOVE PORTABLE CONCRETE BARRIER.
 PAVE FULL DEPTH ASPHALT WEARING SURFACE TO THE LIMITS SHOWN.



STANLY COUNTY STATION: 14+65.00 -L- STATE OF NORTH CAROLINA SHEET 2 OF 2 STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION STABLEGH 038640 STV ENGINEERS, INC. BRIDGE STAGING STV ENGINEERS, INC. DEPARTMENT OF TRANSPORTATION STV ENGINEERS, INC. BRIDGE STAGING STV ENGINEERS, INC. DEPARTMENT OF TRANSPORTATION STV ENGINEERS, INC. BRIDGE STAGING STV ENGINEERS, INC. DEPARTMENT OF TRANSPORTATION STV ENGINEERS, INC. BRIDGE STAGING NC License Number F-0991 REVISIONS NAL UNLESS ALL SHEET NO. ATURES COMPLETED SHEET NO.		PROJEC	CT NO.	<u>178</u>	P.10.R.	103			
SHEET 2 OF 2 SHEET 2 OF 2 STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH BRIDGE STAGING PLAN STV ENGINEERS, INC. 100 STV ENGINERS, INC. 100 STV ENGINEERS, INC. 100 STV ENGI			STAN	LY	CC				
STATE OF NORTH CAROLINA DOG STORE MG INFERIOR NG INFERIOR NG INFERIOR BRIDGE STAGING BRIDGE STAGING PLAN STV ENGINEERS, INC. 900 West Trade St., Suite 715 Charlotte, NC 28202 NC License Number F-0991 REVISIONS SHEET NO. STU ENGINEERD NAL UNLESS ALL NAL UNLESS ALL NUMERS ALL		STATI	ON:	14+65	.00 -L				
DEPARTMENT OF TRANSPORTATION RALEIGH DEPARTMENT OF TRANSPORTATION RALEIGH DEPARTMENT DEPART		SHEET 2 0	F 2						
ATUNESS ALL ATUNESS ALL ATUNESS ALL ATUNESS ALL MOL BY: DATE: NO. BY: DATE: SHEET STACE BRIDGE STAGING PLAN BRIDGE STAGING PLAN BRIDGE STAGING PLAN BRIDGE STAGING PLAN BRIDGE STAGING PLAN SHEET NO. SHEET NO. SHEET STACE SHEET STACE S	Docusion Cost Decusion Cost De		TION						
100 900 West Trade St., Suite 715 Charlotte, NC 28202 NC License Number F-0991 REVISIONS SHEET NO. ENT NOT CONSIDERED NAL UNLESS ALL ATUDES COMPLETED No. BY: DATE: NO. BY: DATE: SHEETS SHEET S	SLEY JOURNAU								
ENT NOT CONSIDERED No. BY: DATE: No. BY: DATE: S-4 NAL UNLESS ALL 1 3 TOTAL	200 900 West Trade St., Suite 715 Charlotte, NC 28202								
NAL UNLESS ALL ATUDES COMPLETED 1 1 3 SHEETS				i I	I				
			DATE:		DATE:				

								STRENGTH I LIMIT STATE										SERVICE III LIMIT STATE										
										SIRE	NGTH		111 5	AIE				SE	RVICE		LIMI	I SIA						
								-		MOMENT					SHEAR						MOMENT							
LEVEL		VEHICLE	WEIGHT (W) (TONS)	CONTROLLING LOAD RATING	MINIMUM RATING FACTORS (RF)	TONS = W X RF	LIVELOAD FACTORS	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (f+)	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (f†)	LIVELOAD FACTORS	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF					
		HL-93(Inv)	N/A	1	1.205		1.75	0.271	1.59	50′	EL	24.482	0.616	1.2	50′	EL	4.896	0.80	0.271	1.46	50′	EL	24.48					
DESIGN		HL-93(0pr)	N⁄A		1.562		1.35	0.271	2.06	50′	EL	24.482	0.616	1.56	50′	EL	4.896	N/A										
LOAD RATING		HS-20(Inv)	36.000	2	1.434	51.614	1.75	0.271	1.97	50′	EL	24.482	0.616	1.43	50′	EL	4.896	0.80	0.271	1.81	50′	EL	24.48					
		HS-20(0pr)	36.000		1.859	66.906	1.35	0.271	2.56	50′	EL	24.482	0.616	1.86	50′	EL	4.896	N/A										
		SNSH	13.500		3.678	49.655	1.4	0.271	5.02	50′	EL	24.482	0.616	4	50′	EL	4.896	0.80	0.271	3.68	50′	EL	24.48					
		SNGARBS2	20.000		2.905	58.101	1.4	0.271	3.97	50′	EL	24.482	0.616	2.93	50′	EL	4.896	0.80	0.271	2.91	50′	EL	24.48					
							SNAGRIS2	22.000		2.748	60.456	1.4	0.271	3.83	50′	EL	19.586	0.616	2.75	50′	EL	4.896	0.80	0.271	2.81	50′	EL	24.48
		SNCOTTS3	27.250		1.835	49.998	1.4	0.271	2.5	50′	EL	24.482	0.616	2.01	50′	EL	4.896	0.80	0.271	1.83	50′	EL	24.48					
	l s	SNAGGRS4	34.925		1.595	55.714	1.4	0.271	2.18	50′	EL	24,482	0.616	1.72	50′	EL	4.896	0.80	0.271	1.60	50′	EL	24.48					
		SNS5A	35.550		1.556	55.303	1.4	0.271	2.12	50′	EL	24.482	0.616	1.77	50′	EL	4.896	0.80	0.271	1.56	50′	EL	24.48					
		SNS6A	39.950		1.455	58.112	1.4	0.271	1.99	50′	EL	24.482	0.616	1.64	50′	EL	4.896	0.80	0.271	1.45	50′	EL	24.48					
LEGAL		SNS7B	42.000		1.386	58.224	1.4	0.271	1.89	50′	EL	24.482	0.616	1.65	50′	EL	4.896	0.80	0.271	1.39	50′	EL	24.48					
LOAD RATING		TNAGRIT3	33.000		1.782	58.809	1.4	0.271	2.43	50′	EL	24.482	0.616	1.94	50′	EL	4.896	0.80	0.271	1.78	50′	EL	24.48					
		TNT4A	33.075		1.798	59.458	1.4	0.271	2.45	50′	EL	24.482	0.616	1.86	50′	EL	4.896	0.80	0.271	1.80	50′	EL	24.48					
		TNT6A	41.600		1.497	62.293	1.4	0.271	2.04	50′	EL	24.482	0.616	1.8	50′	EL	4.896	0.80	0.271	1.50	50′	EL	24.48					
	TST	TNT7A	42.000		1.52	63.842	1.4	0.271	2.08	50′	EL	24.482	0.616	1.67	50′	EL	4.896	0.80	0.271	1 . 52	50′	EL	24.48					
		TNT7B	42.000		1.585	66.559	1.4	0.271	2.16	50′	EL	24.482	0.616	1.59	50′	EL	4.896	0.80	0.271	1.58	50′	EL	24.48					
		TNAGRIT4	43.000		1.504	64.667	1.4	0.271	2.05	50′	EL	24.482	0.616	1.53	50′	EL	4.896	0.80	0.271	1.50	50′	EL	24.48					
		TNAGT5A	45.000		1.405	63.217	1.4	0.271	1.92	50′	EL	24.482	0.616	1.56	50′	EL	4.896	0.80	0.271	1.40	50′	EL	24.48					
		TNAGT5B	45.000	3	1.376	61.936	1.4	0.271	1.88	50′	EL	24.482	0.616	1.45	50′	EL	4.896	0.80	0.271	1.38	50′	EL	24.48					

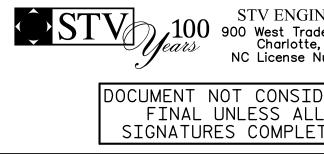


 $\begin{pmatrix} 1 \\ 2 \end{pmatrix}$ $\langle 3 \rangle$

LRFR SUMMARY

FOR SPAN 'A'

+



LOAD FACTORS:

DESIGN LOAD RATING	LIMIT STATE	γ_{DC}	$\gamma_{\sf DW}$
	STRENGTH I	1.25	1.50
FACTORS	SERVICE III	1.00	1.00

NOTES:

БR

٦

AENT

сомм

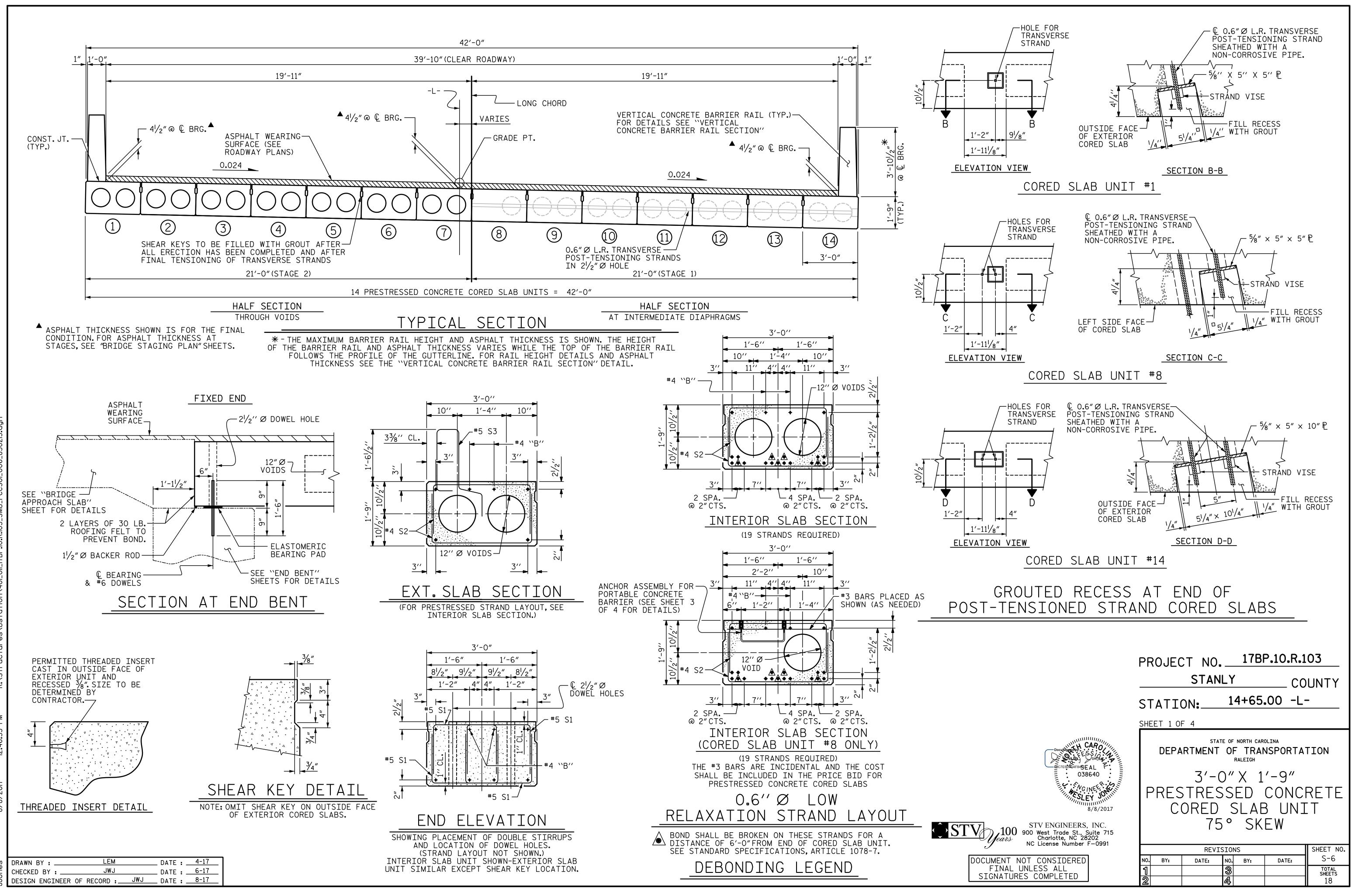
MINIMUM RATING FACTORS ARE BASED ON THE STRENGTH I AND SERVICE III LIMIT STATES. ALLOWABLE STRESSES FOR SERVICE III LIMIT STATE ARE AS REQUIRED FOR DESIGN.

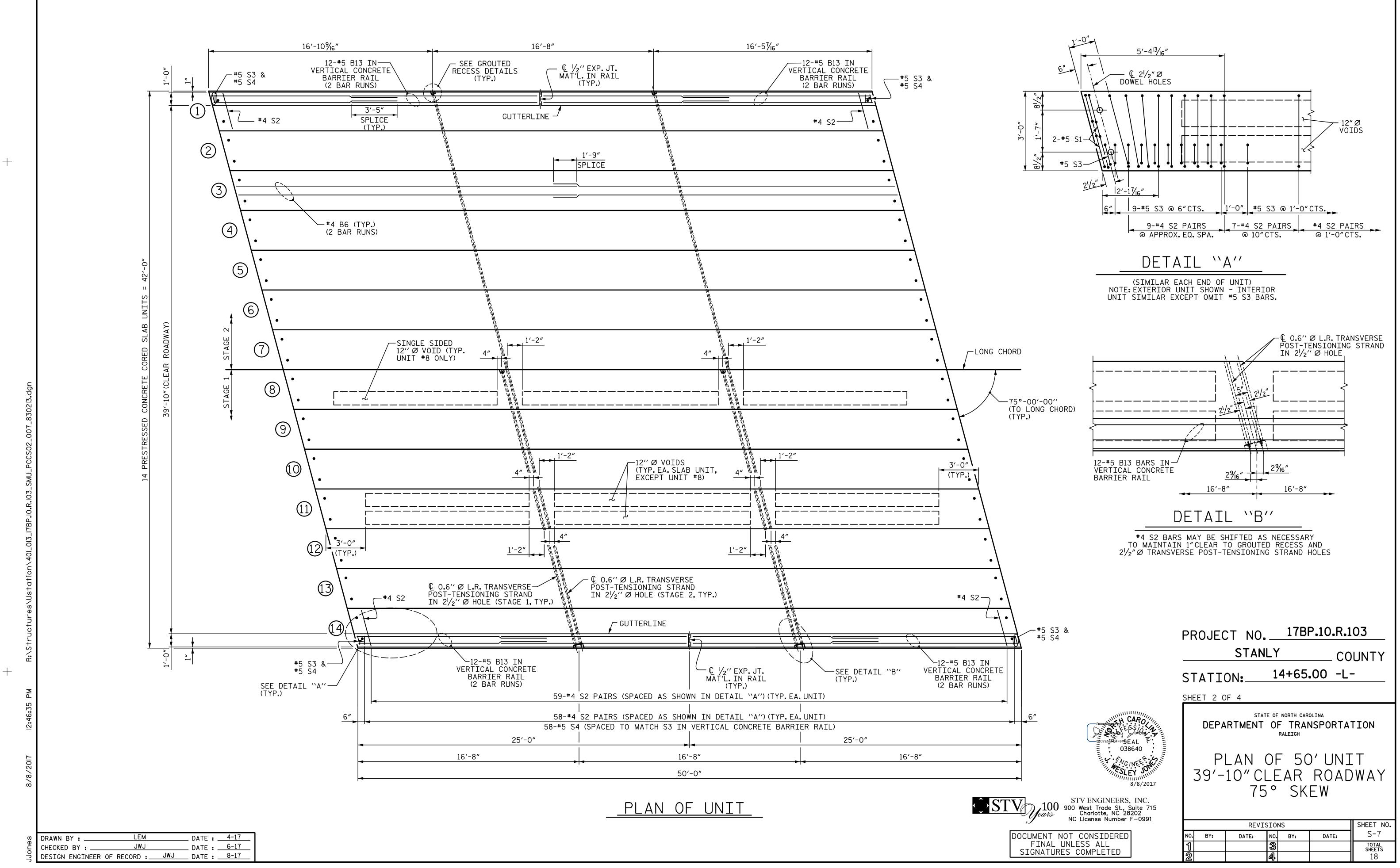
COMMENTS:

- 1. 2.
- 3.
- 4.

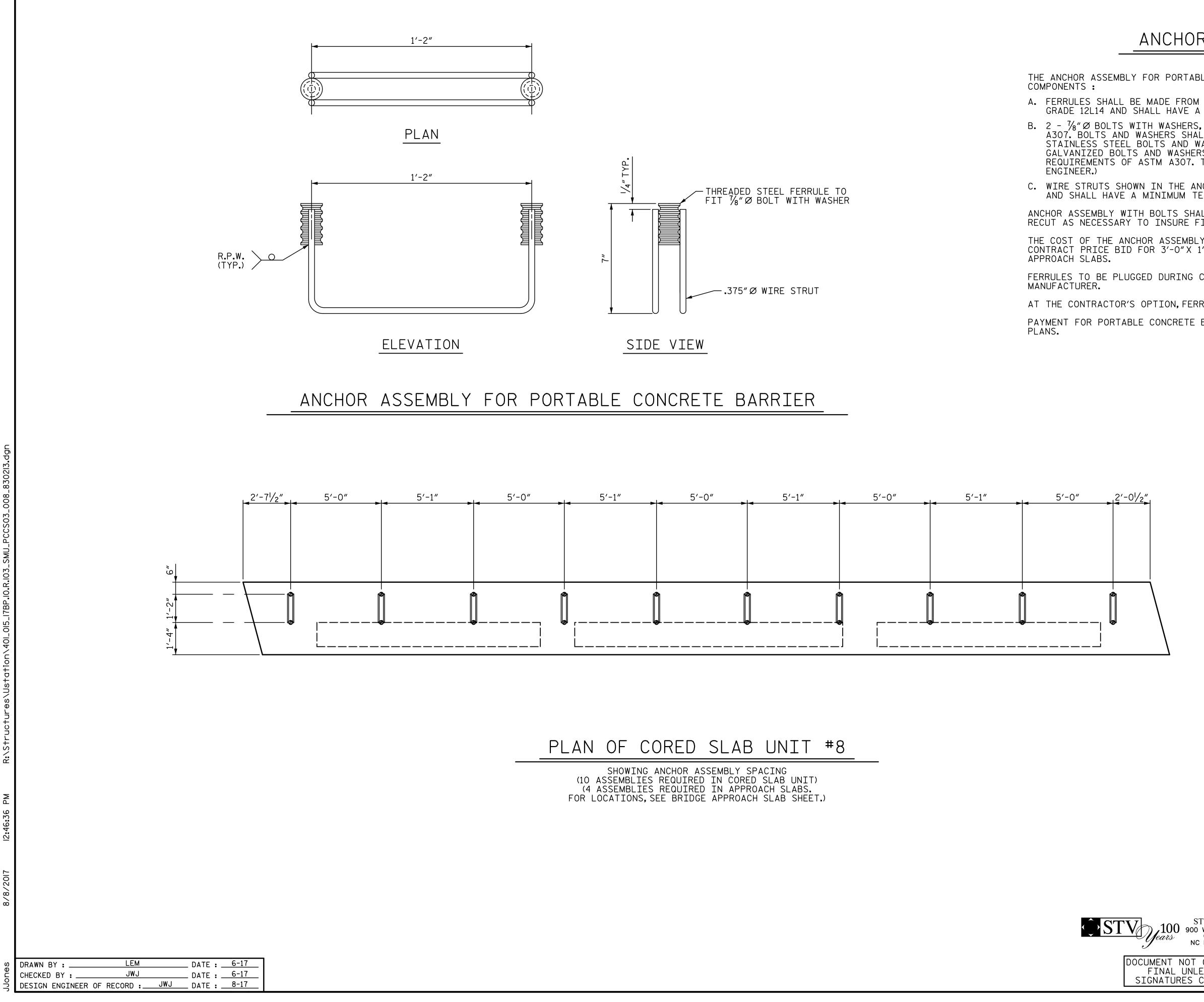
•	(#) CONTROLLING LOAD RATING
•	1 DESIGN LOAD RATING (HL-93)
	2 DESIGN LOAD RATING (HS-20)
•	$\sqrt{3}$ LEGAL LOAD RATING $**$
	** SEE CHART FOR VEHICLE TYPE
	GIRDER LOCATION
	I - INTERIOR GIRDER EL - EXTERIOR LEFT GIRDER ER - EXTERIOR RIGHT GIRDER

	PROJEC	PROJECT NO. 178P.10.R.103								
		STAN	LY	CO	UNTY					
	STATI	STATION: 14+65.00 -								
CARO CARO CONSERVATE CONSERVATE CONSERVATE CONSERVATION CONSERVATI	LR 50′	STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTA RALEIGH STANDARD LRFR SUMMARY F 50' CORED SLAB U 75° SKEW (NON-INTERSTATE TRAF								
		REVIS	SIONS		SHEET NO.					
ONSIDERED S ALL	NO. BY:	DATE:	NO. BY:	DATE:	S-5					
MPLETED	1 2		3 4		total sheets 18					
	STD	. NO. 21L	RFR1_758	105S_50L	_					





+

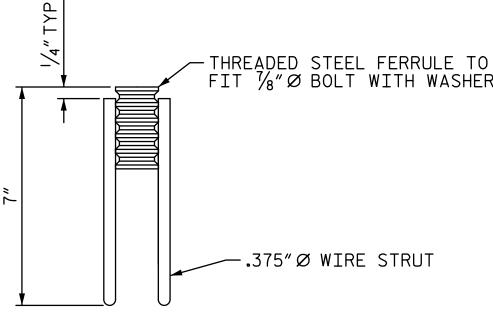


ANCHOR ASSEMBLY WITH BOLTS SHALL BE ASSEMBLED IN THE SHOP. BOLT THREADS MAY BE RECUT AS NECESSARY TO INSURE FIT.

THE COST OF THE ANCHOR ASSEMBLY COMPLETE IN PLACE SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR 3'-O"X 1'-9" PRESTRESSED CONCRETE CORED SLABS AND BRIDGE

FERRULES TO BE PLUGGED DURING CASTING OF CORED SLAB UNIT AS RECOMMENDED BY THE

AT THE CONTRACTOR'S OPTION, FERRULES WITH OPEN OR CLOSED ENDS MAY BE USED. PAYMENT FOR PORTABLE CONCRETE BARRIER RAIL IS INCLUDED IN THE TRAFFIC CONTROL



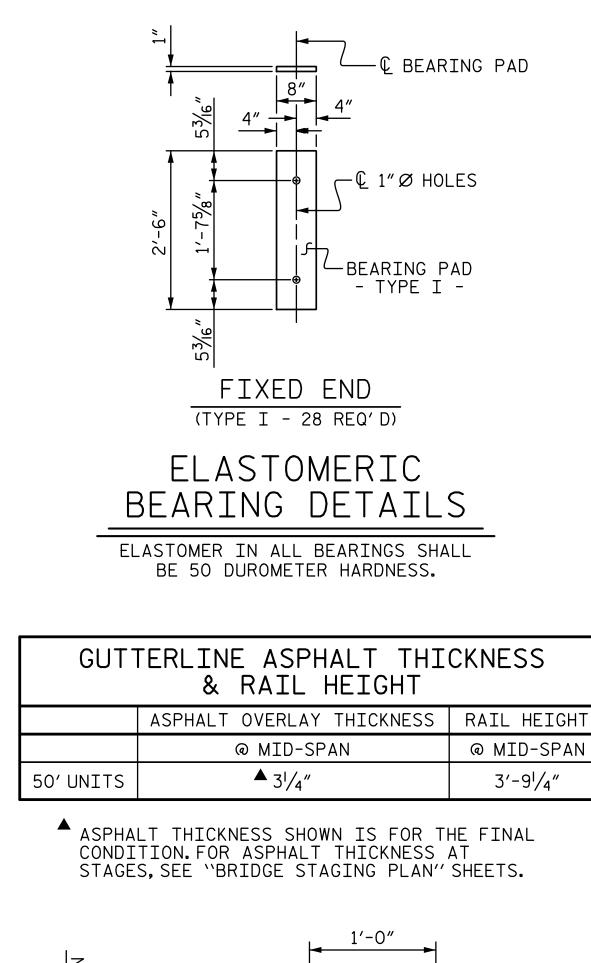
ANCHOR ASSEMBLY NOTES

THE ANCHOR ASSEMBLY FOR PORTABLE CONCRETE BARRIER SHALL CONSIST OF THE FOLLOWING

A. FERRULES SHALL BE MADE FROM STEEL MEETING THE REQUIREMENTS OF AASHTO M169, GRADE 12L14 AND SHALL HAVE A MINIMUM LENGTH OF THREADS OF $2^{1}/_{2}$ ". B. 2 - ⁷/₈" Ø BOLTS WITH WASHERS, BOLTS SHALL CONFORM TO THE REQUIREMENTS OF ASTM A307. BOLTS AND WASHERS SHALL BE GALVANIZED. (AT THE CONTRACTOR'S OPTION, STAINLESS STEEL BOLTS AND WASHERS MAY BE USED AS AN ALTERNATE FOR THE ⁷/₈" Ø GALVANIZED BOLTS AND WASHERS. THEY SHALL CONFORM TO OR EXCEED THE MECHANICAL DEPUTY OF ACTIVE ASTA THE HEE OF THE OF AN ATTENNATE OF ADDRESS THE REQUIREMENTS OF ASTM A307. THE USE OF THIS ALTERNATE SHALL BE APPROVED BY THE

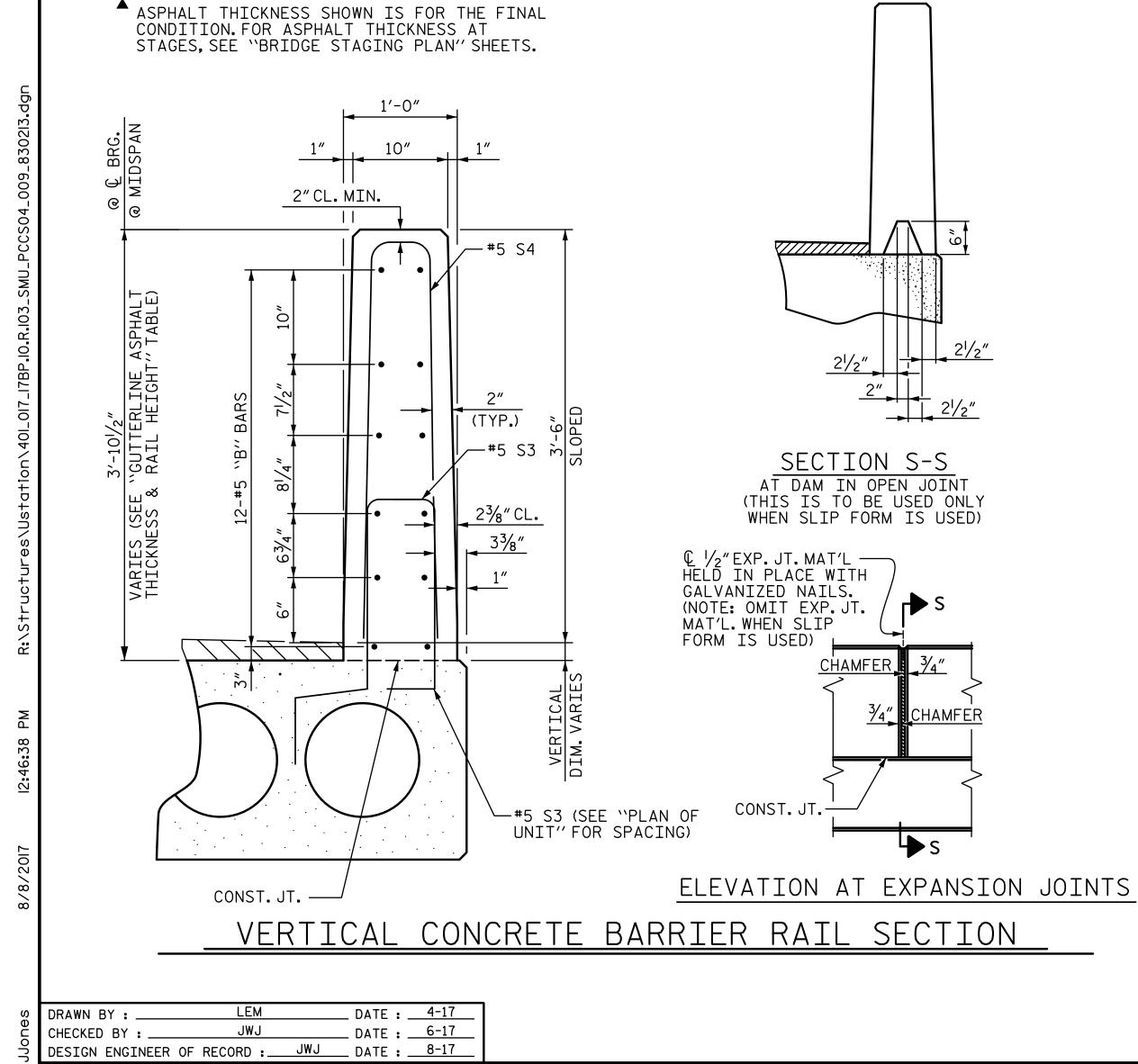
C. WIRE STRUTS SHOWN IN THE ANCHOR ASSEMBLY DETAIL ARE MINIMUM ALLOWABLE SIZE AND SHALL HAVE A MINIMUM TENSILE STRENGTH OF 100,000 P.S.I.

	PROJEC	CT NO.	<u>178</u>	P.10.R.1	.03
		STAN	LY	CO	UNTY
	STATI	ON:	14+65	.00 -L·	-
	SHEET 3 0	F 4			
V ENGINEERS, INC. West Trade St., Suite 715 Charlotte, NC 28202 License Number F-0991	PRES	RTMENT 3'-C STRES ORED 75	of north car OF TRAI raleigh SSED SLAE S SK	nsporta .'-9'' CONCI 3 UNI	RETE T
			SIONS		SHEET NO. S-8
CONSIDERED SS ALL COMPLETED	NO. BY: 1 2	DATE:	NO. ВҮ: 3 4	DATE:	TOTAL SHEETS 18

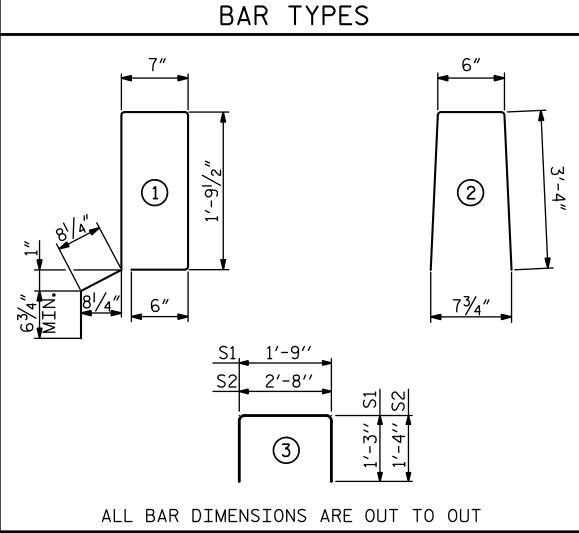


CORED SLABS REQUIRED								
STAGE NUMBER		NUMBER	LENGTH	TOTAL LENGTH				
	EXTERIOR C.S.	1	50'-0″	50'-0"				
1	INTERIOR C.S.	6	50'-0"	300'-0"				
	TOTAL	7		350′-0″				
	EXTERIOR C.S.	1	50'-0"	50′-0″				
2	INTERIOR C.S.	6	50′-0″	300'-0″				
	TOTAL	7		350′-0″				

	BI	LL OF	ΜΑΤΕ	RIAL F	OR ONE	50' CORE	ED SLAB	UNIT			
EXTERIOR UNIT INTERIOR UNIT SINGLE VOID UNIT											
BAR	NUMBER	SIZE	TYPE	LENGTH	WEIGHT	LENGTH	WEIGHT	LENGTH	WEIGHT		
B6	4	#4	STR	25′-9″	69	25′-9″	69	25'-9"	69		
S1	8	#5	3	4'-3"	35	4'-3"	35	4'-3"	35		
S2	118	#4	3	5′-4″	420	5'-4"	420	5'-4"	420		
* S3	60	#5	1	5′-11″	370						
REINFO	DRCING 3	STEEL	LBS	.	524		524		524		
	Y COATE										
REIN	FORCINC	<u>, steel</u>	LBS		370						
6500 P.S.I. CONCRETE CU. YDS. 7.4 7.4 8.5								8.5			
0.6″Ø	L.R. STR	ANDS	Nc).	19		19		19		

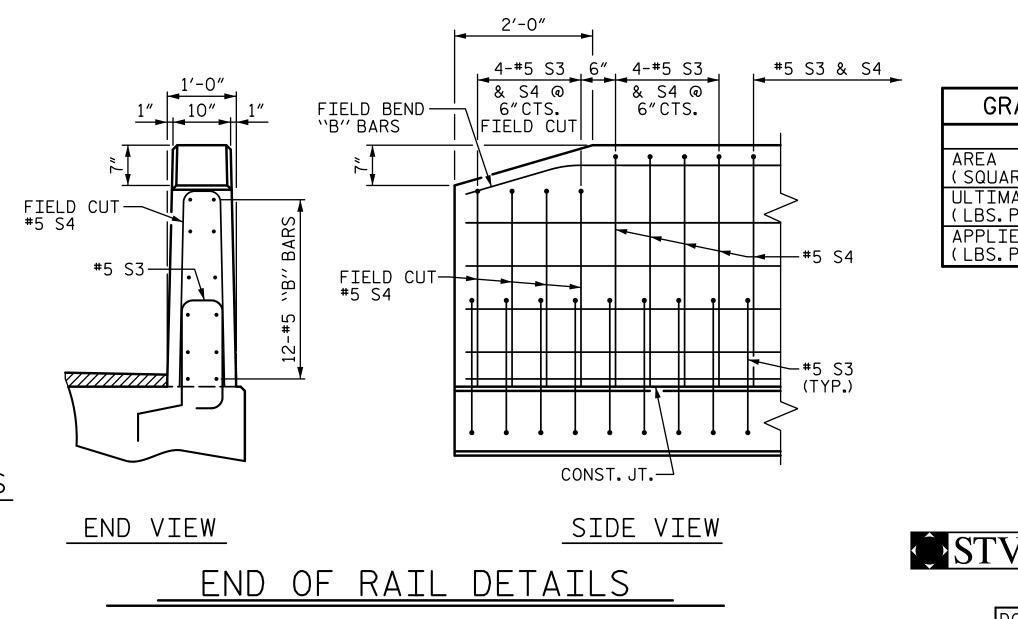


+

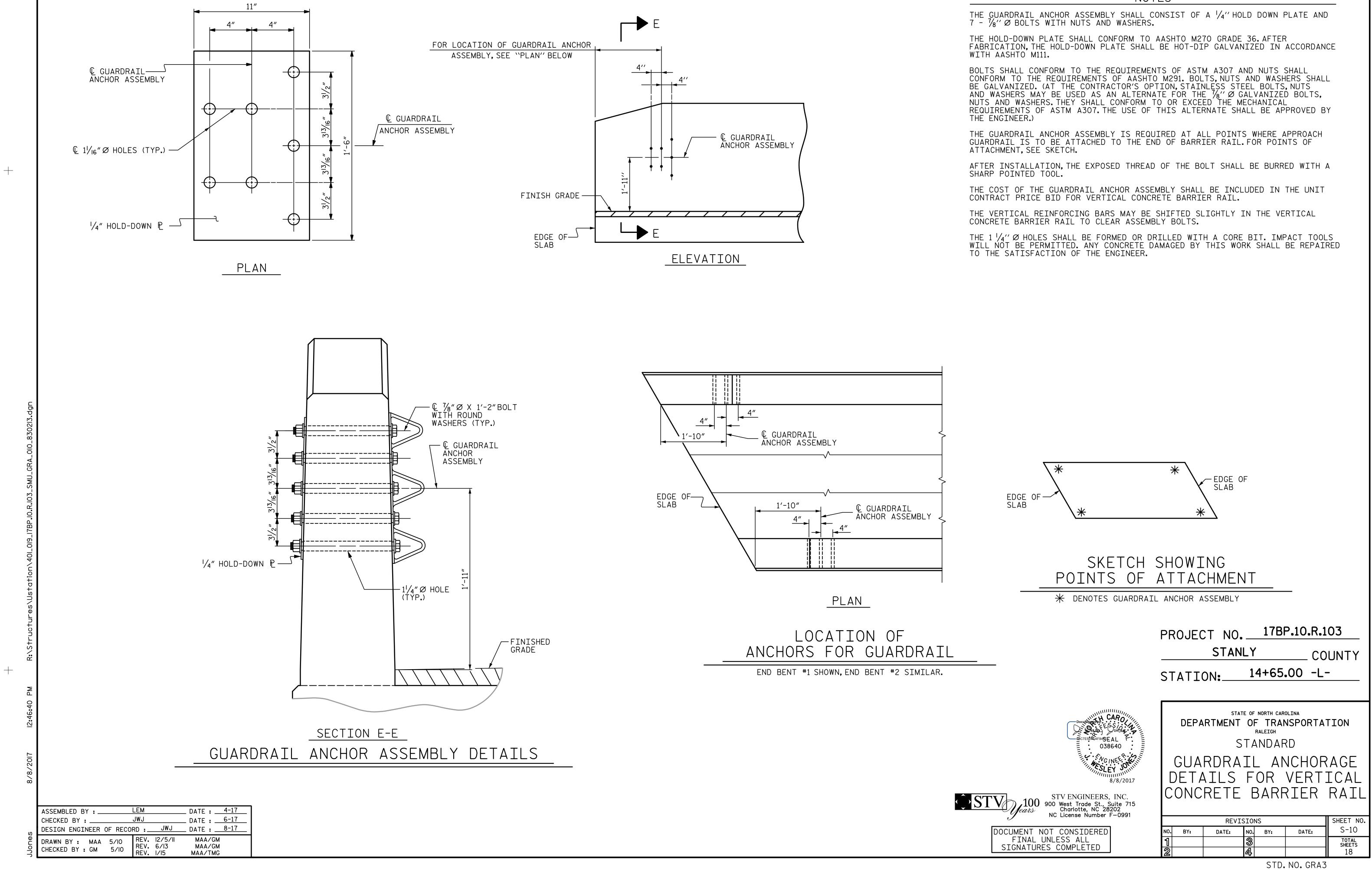


DEAD LOAD DEFLECTION AN	ND CAMBER
	3'-0"× 1'-9"
50' CORED SLAB UNIT	0.6″ØL.R. STRAND
CAMBER (SLAB ALONE IN PLACE)	1 ∕2″ ♦
DEFLECTION DUE TO SUPERIMPOSED DEAD LOAD	3∕8″ ∳
FINAL CAMBER	1 ∕8″ ♦
WE THELLER FUTUE WEADTHE SUDE	

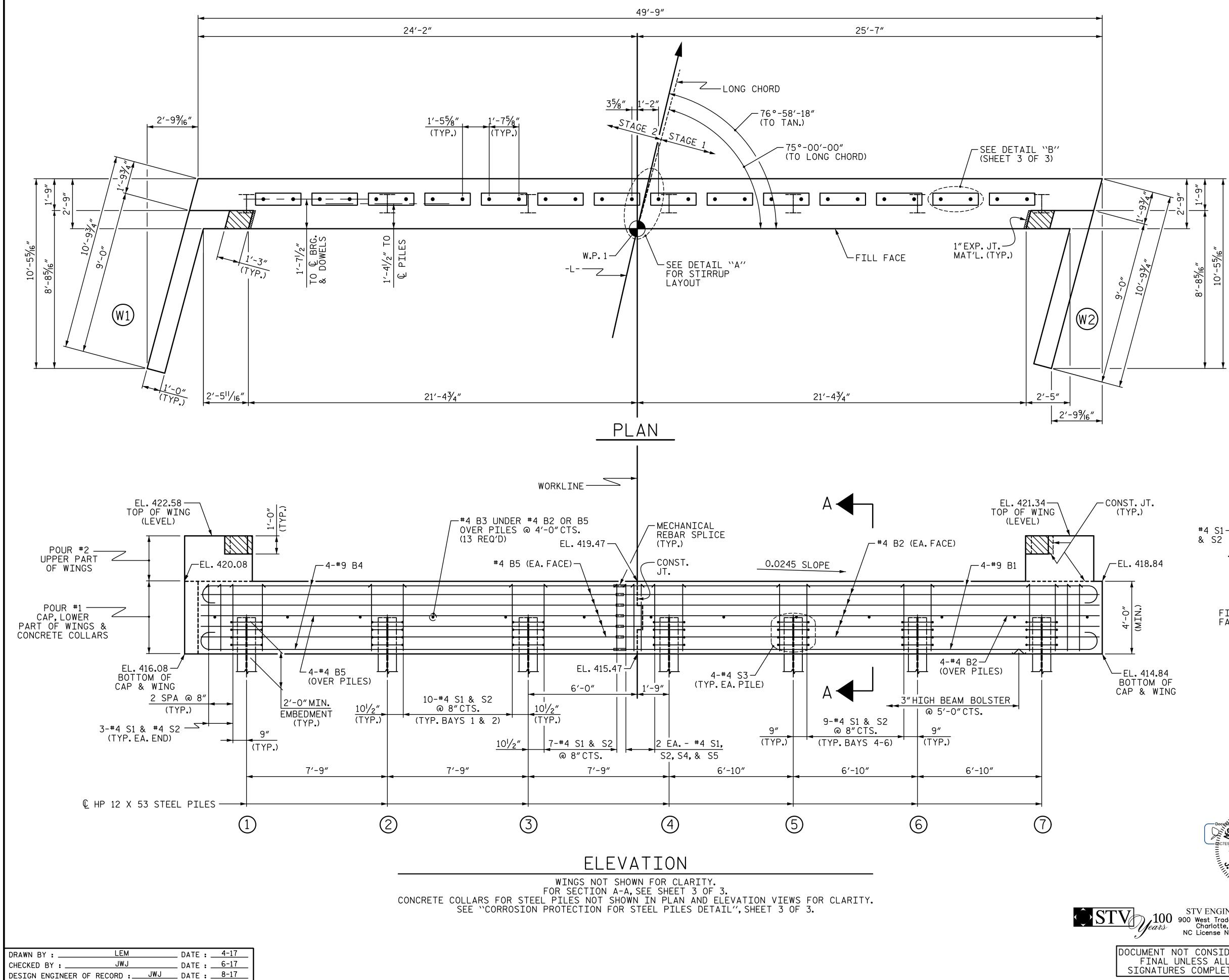
	DAN THES	NOTES
JIRED NGTH TOTAL LENGTH		ALL PRESTRESSING STRANDS SHALL BE 7-WIRE LOW RELAXATION GRADE 270 STRANDS AND SHALL CONFORM TO AASHTO M203 EXCEPT FOR SAMPLING REQUIREMENTS WHICH SHALL BE IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.
0'-0" 50'-0" 0'-0" 300'-0"	(1) ^{*,2} / ₁₆₋ , (2) 3'-4"	ALL REINFORCING STEEL CAST WITH THE CORED SLAB SECTIONS SHALL BE GRADE 60 AND SHALL BE INCLUDED IN THE UNIT PRICE BID FOR PRESTRESSED CONCRETE CORED SLABS.
		RECESSES FOR TRANSVERSE STRANDS SHALL BE GROUTED AFTER THE TENSIONING OF THE STRANDS.
0'-0" <u>300'-0"</u> <u> </u>		THE $2^{1/2}$ $^{\prime\prime}$ Ø dowel holes at fixed ends of slab sections shall be filled with non-shrink grout.
	<u>S1</u> 1'-9''	THE BACKER RODS SHALL CONFORM TO THE REQUIREMENTS OF TYPE M BOND BREAKER.SEE SECTION 1028 OF THE STANDARD SPECIFICATIONS.
RED SLAB UNIT RIOR UNIT SINGLE VOID UNIT H WEIGHT LENGTH WEIGHT 7 69 25'-9" 69	S2 2'-8'' IS IS N N	WHEN CORED SLABS ARE CAST, AN INTERNAL HOLD-DOWN SYSTEM SHALL BE EMPLOYED TO PREVENT VOIDS FROM RISING OR MOVING SIDEWAYS. AT LEAST SIX WEEKS PRIOR TO CASTING CORED SLABS, THE CONTRACTOR SHALL SUBMIT TO THE ENGINEER FOR REVIEW AND COMMENT, DETAILED DRAWINGS OF THE PROPOSED HOLD-DOWN SYSTEM. IN ADDITION TO STRUCTURAL DETAILS, LOCATION AND SPACING OF THE HOLD-DOWNS SHALL BE INDICATED.
<i>"</i> 35 4′-3 <i>"</i> 35	ALL BAR DIMENSIONS ARE OUT TO OUT	ALL REINFORCING STEEL IN THE VERTICAL CONCRETE BARRIER RAIL SHALL BE EPOXY COATED.
<u>420 5'-4" 420</u>		PRESTRESSING STRANDS SHALL BE CUT FLUSH WITH THE CORED SLAB UNIT ENDS.
524 524		APPLY EPOXY PROTECTIVE COATING TO CORED SLAB UNIT ENDS.
7.4 8.5	DEAD LOAD DEFLECTION AND CAMBER 3'-0"×1'-9" 50' CORED SLAB UNIT 0.6"ØL.R. STRAND CAMBER (SLAB ALONE IN PLACE) 1½" DEFLECTION DUE TO	GROOVED CONTRACTION JOINTS, 1/2" IN DEPTH, SHALL BE TOOLED IN ALL EXPOSED FACES OF THE BARRIER RAIL AND IN ACCORDANCE WITH ARTICLE 825-10(B) OF THE STANDARD SPECIFICATIONS. A CONTRACTION JOINT SHALL BE LOCATED AT EACH THIRD POINT BETWEEN BARRIER RAIL EXPANSION JOINTS. ONLY ONE CONTRACTION JOINT IS REQUIRED AT MIDPOINT OF BARRIER RAIL SEGMENTS LESS THAN 20 FEET IN LENGTH AND NO CONTRACTION JOINTS ARE REQUIRED FOR THOSE SEGMENTS LESS THAN 10 FEET IN LENGTH.
	DEFLECTION DUE TO SUPERIMPOSED DEAD LOAD FINAL CAMBER 1½8″	ALLOWED. THE TRANSFER OF LOAD FROM THE ANCHORAGES TO THE CORED SLAB UNIT
	** INCLUDES FUTURE WEARING SURFACE	SHALL BE DONE WHEN THE CONCRETE HAS REACHED A COMPRESSIVE STRENGTH OF NOT LESS THAN THE REQUIRED STRENGTH SHOWN IN THE ``CONCRETE RELEASE STRENGTH'' TABLE.
		FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.
BILL OF MATERIAL FO	R VERTICAL CONCRETE BARRIER RAIL	THE PERMITTED THREADED INSERTS ARE DETAILED AS AN OPTION FOR THE CONTRACTOR TO ATTACH FALSEWORK AND FORMWORK DURING CONSTRUCTION.
BAR BARS PER PAIR OF EXTERI 50' UNIT	IOR UNITS TOTAL NO. SIZE TYPE LENGTH WEIGHT	THE PERMITTED THREADED INSERTS IN THE EXTERIOR UNITS SHALL BE SIZED BY THE CONTRACTOR, SPACED AT 4'-O"CENTERS AND GALVANIZED IN ACCORDANCE WITH SECTION 1076 OF THE STANDARD SPECIFICATIONS.
*B13 96	96 # 5 STR 14'-2" 1418	STAINLESS STEEL THREADED INSERTS MAY BE USED AS AN ALTERNATE. THE PERMITTED THREADED INSERTS SHALL BE GROUTED BY THE CONTRACTOR
* S4 120 * EPOXY COATED REINFORCING STEE	120 #5 2 7'-2" 897 EL LBS. 2315	IMMEDIATELY FOLLOWING REMOVAL OF THE FALSEWORK. THE COST OF THE PERMITTED THREADED INSERTS SHALL BE INCLUDED IN
CLASS AA CONCRETE TOTAL VERTICAL CONCRETE BARRIEF	CU.YDS. 13.3	THE PRICE BID FOR THE PRECAST UNITS.
	2'-0"	CONCRETE RELEASE STRENGTHUNITPSI50' UNITS4900
1" 10" 1" FIELD BEND-	4-#5 S3 6″ 4-#5 S3 & S4 @ 8 S4 @ 6″ CTS. 6″ CTS. G″ CTS. G″ CTS.	DE 270 STRANDS
	GRAD	0.6"ØL.R.
	(SQUARE	INCHES)0.217PROJECT NOIDT.IOT.IOT.IOT.E STRENGTH R STRAND)58,600STANLYCOUNTY
	APPLIED (LBS.PEF	PRESTRESS 43,950 STATION: 14+65.00 -L-
**5 53 FIELD CUT *5 54 *5 54		SHEET 4 OF 4
	#5 S3 (TYP.)	DOCUMENTATION DOCUMENTATION DOCUMENTATION DEPARTMENT OF TRANSPORTATION RALEIGH
		3'-0' X 1'-9'
	CONST. JT.	PRESTRESSED CONCRETE 8/8/2017 CORED SLAB UNIT
END VIEW	SIDE VIEW	STV ENGINEERS, INC. 75° SKEW
<u>END OF RAIL</u>	<u>DETAILS</u>	REVISIONS SHEET NO.
		UMENT NOT CONSIDEREDNO.BY:DATE:NO.BY:DATE:S-9FINAL UNLESS ALL GONATURES COMPLETED13TOTAL SHEETS 18



NOTES



NOTES



+

NOTES

STIRRUPS IN CAP MAY BE SHIFTED AS NECESSARY TO CLEAR DOWELS.

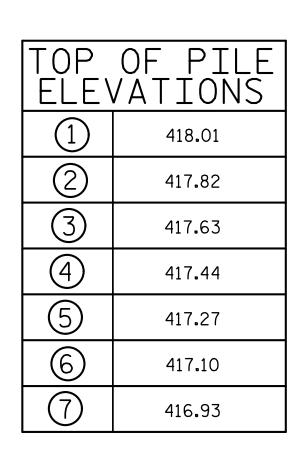
THE CONCRETE IN THE SHADED AREA OF THE WING SHALL BE POURED AFTER THE VERTICAL CONCRETE BARRIER RAIL IS CAST IF SLIP FORMING IS USED.

FOR PILE SPLICE DETAILS, SEE SHEET 3 OF 3.

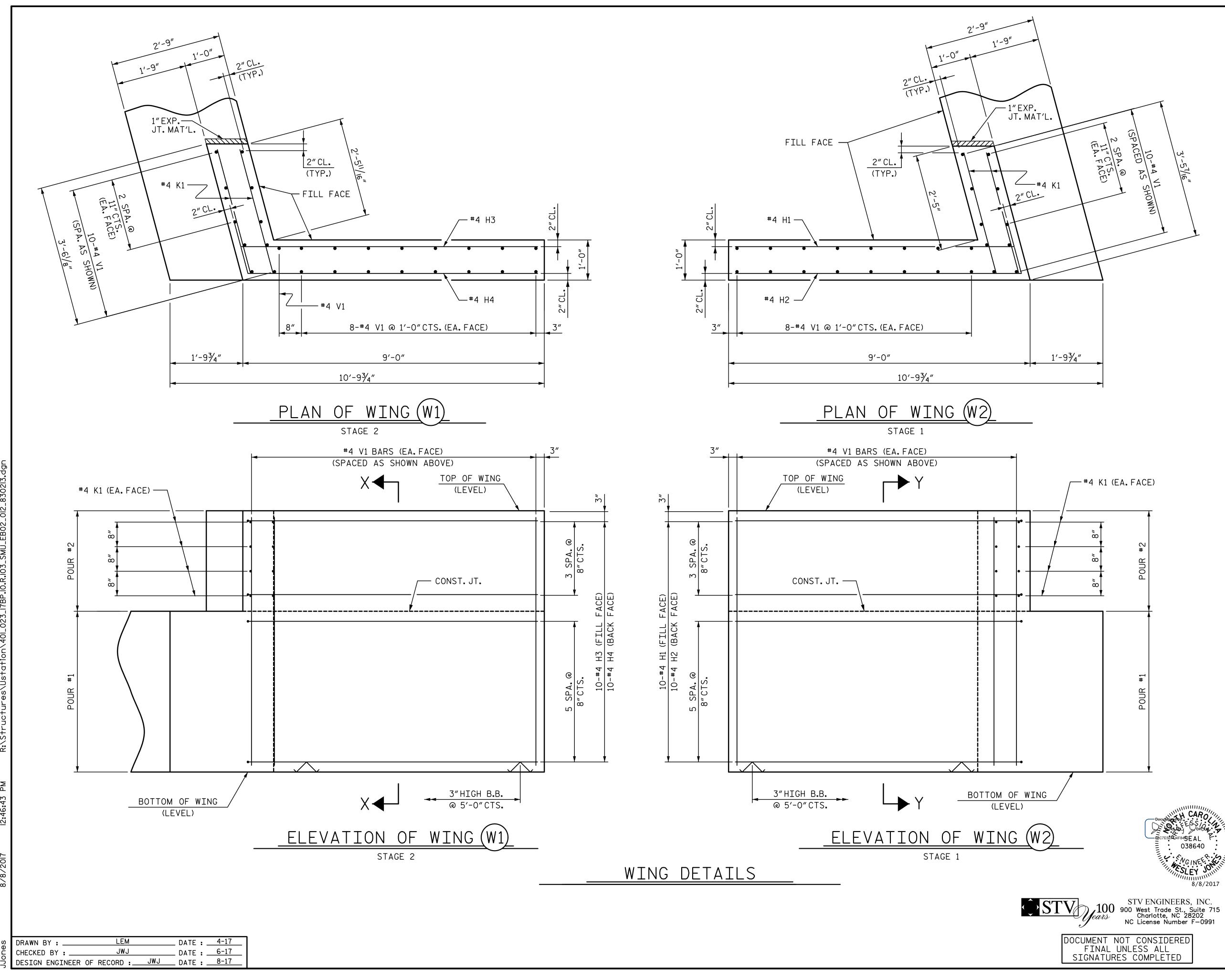
FOR WING DETAILS, SEE SHEET 2 OF 3. FOR CONSTRUCTION JOINT DETAILS, SEE

SHEET 3 OF 3.

FOR MECHANICAL SPLICES, SEE SECTION 425-5(B) OF THE STANDARD SPECIFICATIONS.



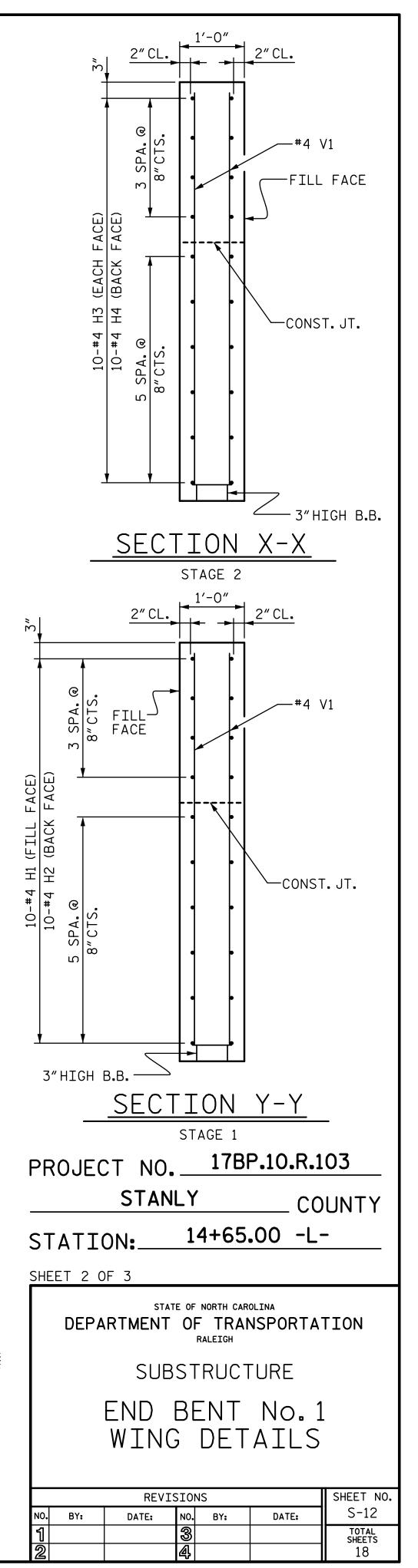
JT. H18.84 F_{ACE}	2 EQUAL SPA. (TYP.) + + + 4 S1 *4 S1 *4 S1 *4 S2 +4 S4 & S5 CONST. JT.
414.84 TOM OF & WING	AIL ``A''
-	PROJECT NO. 17BP.10.R.103 STANLY COUNTY STATION: 14+65.00 -L- SHEET 1 OF 3 3
Docusianto Docusi	STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH SUBSTRUCTURE END BENT No.1
STV ENGINEERS, INC. 900 West Trade St., Suite 715 Charlotte, NC 28202 NC License Number F-0991 JMENT NOT CONSIDERED FINAL UNLESS ALL GNATURES COMPLETED	REVISIONS SHEET NO. NO. BY: DATE: NO. BY: DATE: SHEET NO. 1 3 0. BY: DATE: SHEET NO. 2 4 18 18

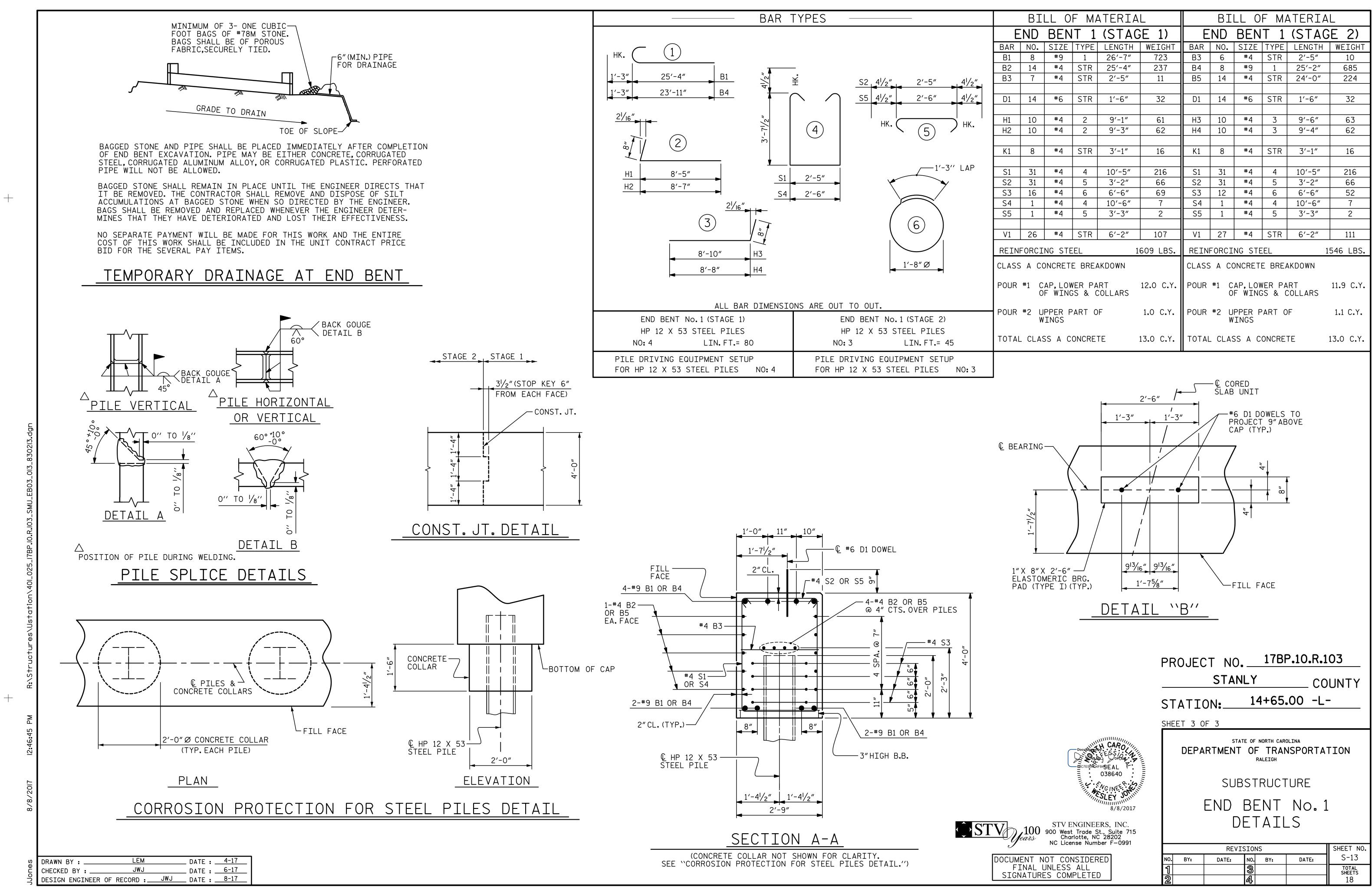


+

م 4F84 038640 8/8/2017

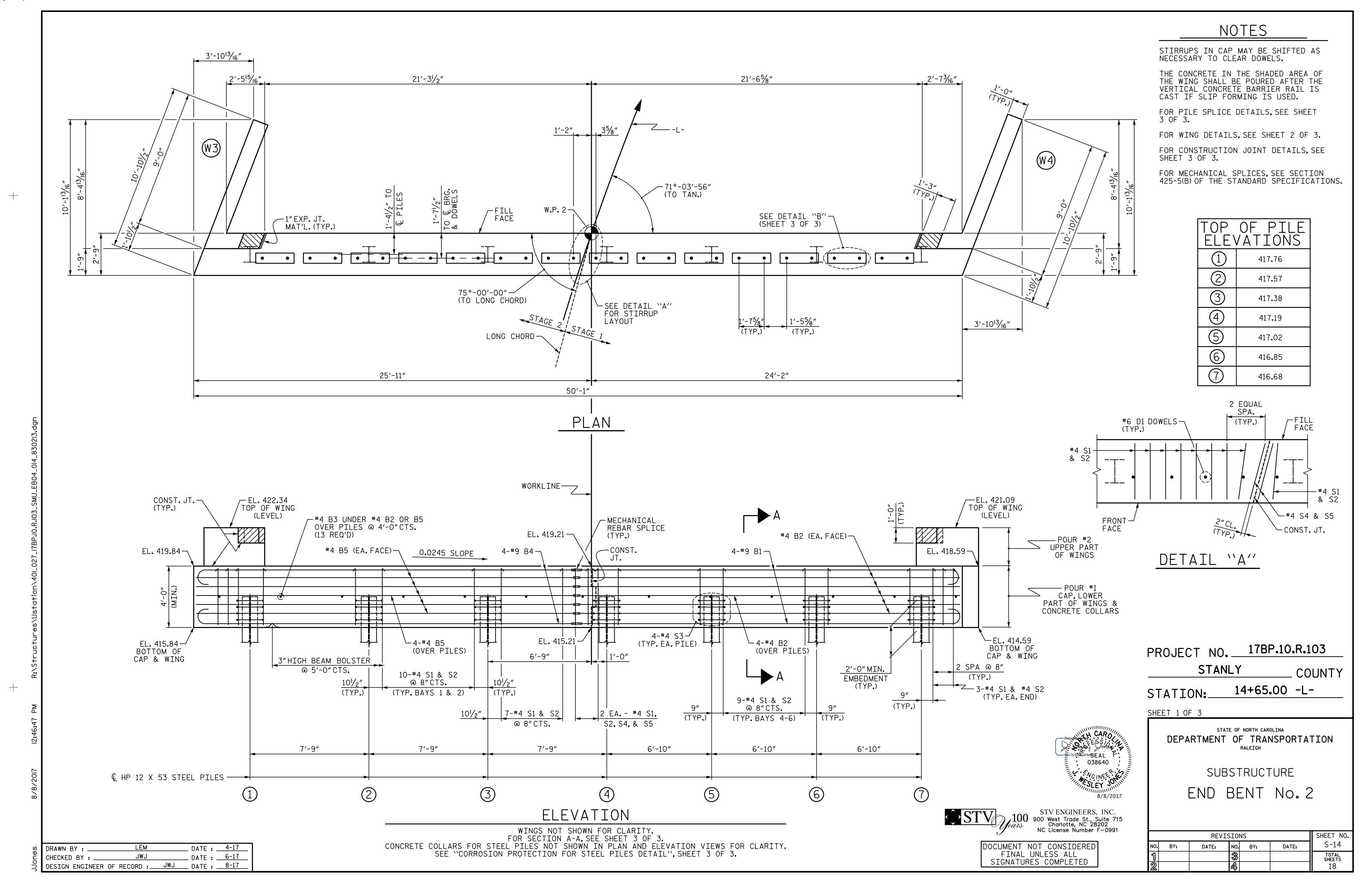
MENT	NOT	CONSIDERED
FINAL	UNL	ESS ALL
GNATU	RES	COMPLETED

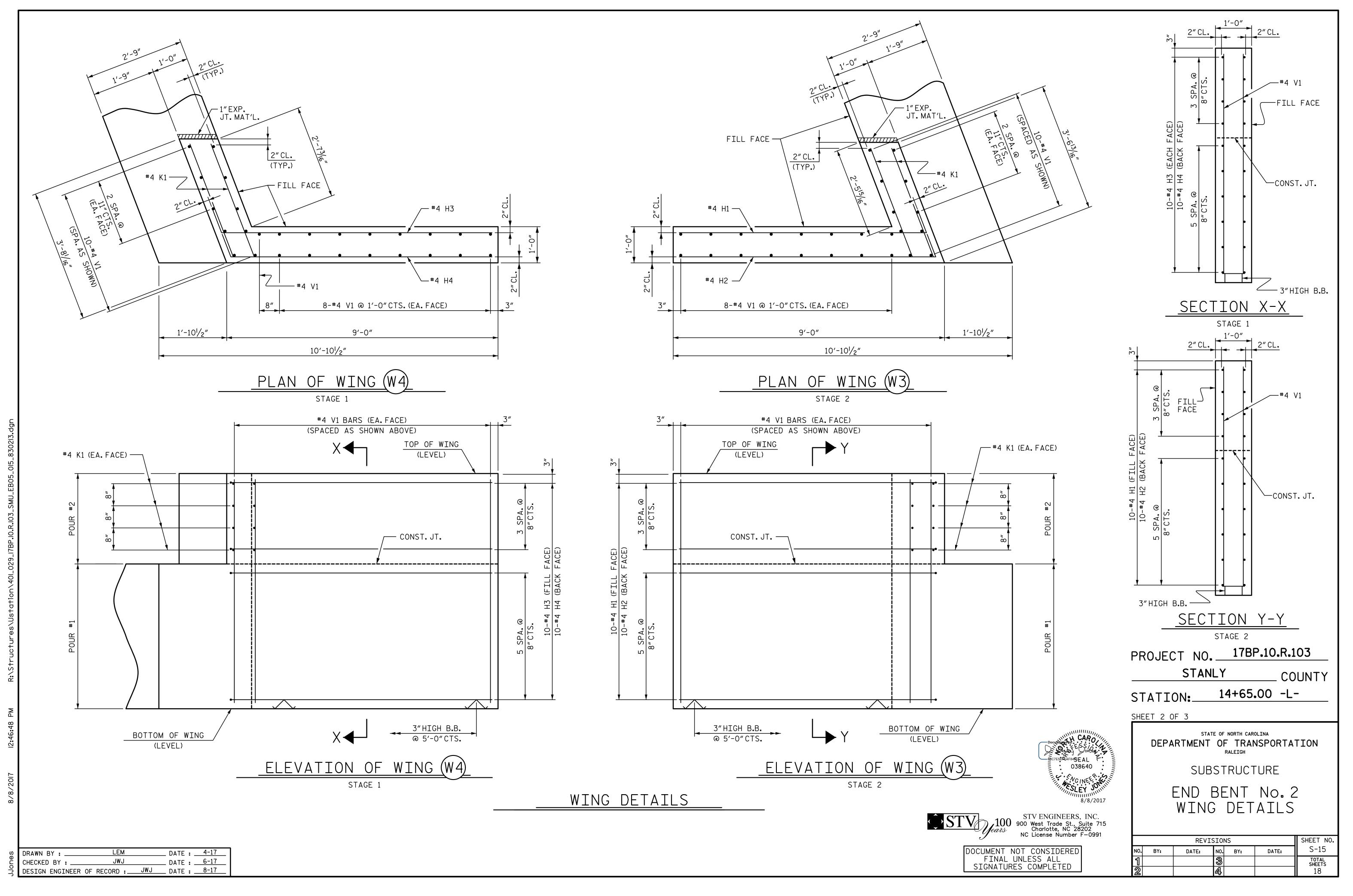


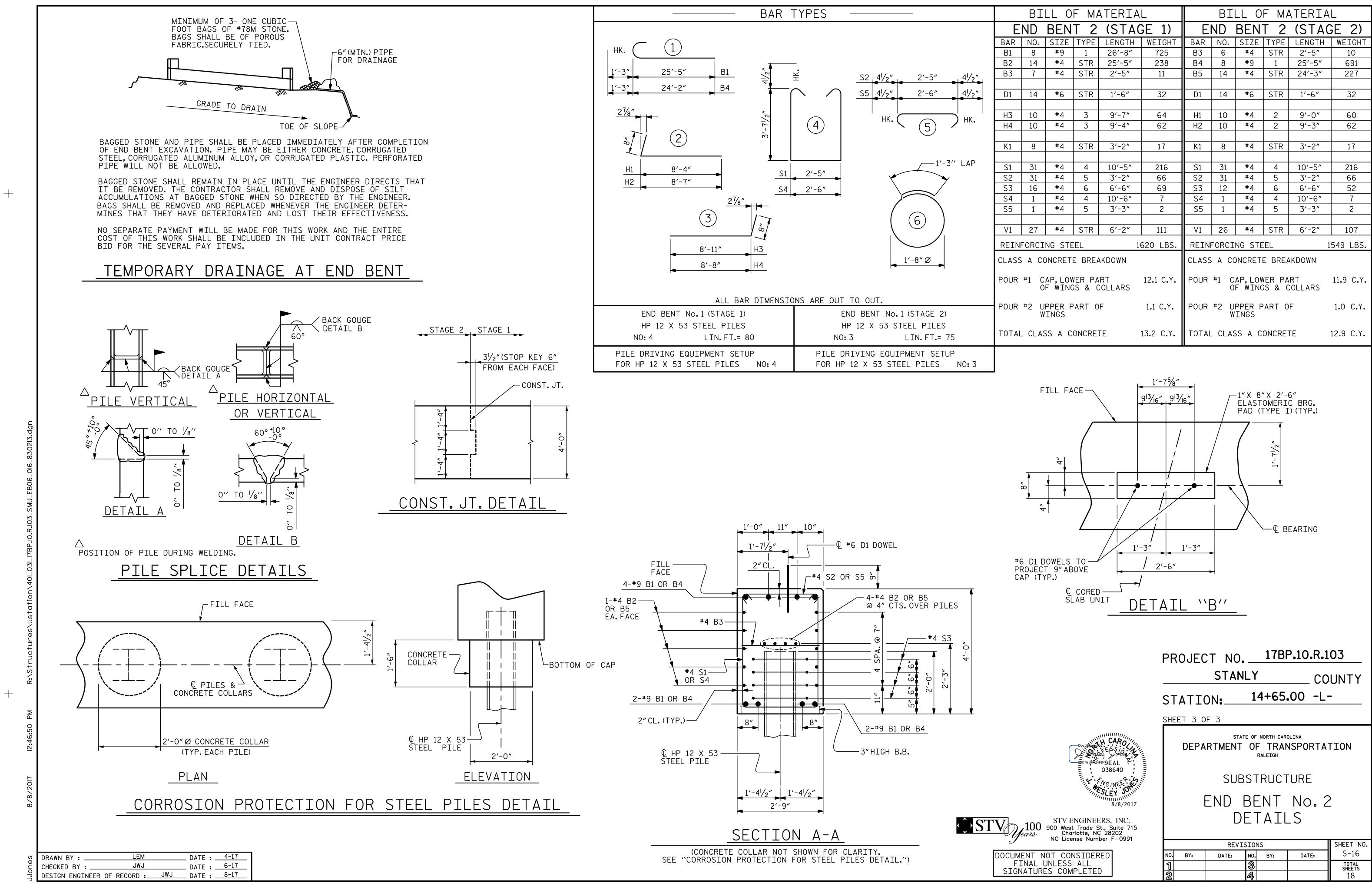


	ΒI	LL O	F MA	ATERIA	L		BI	LL O	F MA	ATERIA	L
EI	ND	BEN	T 1	(STAG	E 1)	E	ND	BEN	T 1	(STAC	SE 2)
2	NO.	SIZE	TYPE	LENGTH	WEIGHT	BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT
	8	#9	1	26′-7″	723	B3	6	#4	STR	2′-5″	10
	14	#4	STR	25'-4″	237	B4	8	#9	1	25′-2″	685
	7	#4	STR	2′-5″	11	B5	14	#4	STR	24'-0"	224
	14	#6	STR	1'-6″	32	D1	14	#6	STR	1'-6″	32
	10	#4	2	9'-1"	61	H3	10	#4	3	9'-6″	63
	10	#4	2	9'-3"	62	H4	10	#4	3	9'-4"	62
	8	#4	STR	3'-1"	16	K1	8	#4	STR	3'-1"	16
-+	74				01.0	<u> </u>	74				01.0
	31	#4	4	10'-5"	216	S1	31	#4	4	10'-5"	216
	31	#4	5	3'-2"	66	S2	31	#4	5	3'-2"	66
_	16	#4	6	6'-6"	69	S3	12	#4	6	6'-6"	52
-+	1	#4 #4	4	10'-6" 3'-3"	7	S4	1	#4	4	10'-6"	7
_	1	#4	5	22.	2	S5	1	#4	5	3'-3"	2
	26	#4	STR	6′-2″	107	V1	27	#4	STR	6'-2"	111
NF	ORCI	NG STE	EL	1	609 LBS.	REIN	FORCI	NG STE	EL	1	.546 LBS.
SS	A CO	ONCRET	E BREA	KDOWN		CLASS	A CO	DNCRET	E BREA	KDOWN	
~ 4	4 1 0			DT.			#1 0			D.T.	
IR #1 CAP,LOWER PART 12.0 C.Y. POUR #1 CAP,LOWER PART 11.9 OF WINGS & COLLARS OF WINGS & COLLARS						11.9 C.Y.					
२ ‡		PPER P	PART C	F	1.0 C.Y.	POUR		PPER P INGS	ART O	F	1.1 C.Y.
AL	CLAS	SS A C	ONCRE	TE	13.0 C.Y.	TOTAL	CLAS	SS A C	ONCRE	TE	13.0 C.Y.
AL	CLAS	SS A C	ONCRE	TE	13.0 C.Y.	TOTAL	CLAS	SS A C	ONCRE	TE	13.0

	PROJEC	CT NO.	17B	P . 10 . R.	.103
		STANL			OUNTY
	STATI	0N:	14+65	.00 -L	
	SHEET 3 C)F 3			
Docustor CARO Docustor FESS Over THE Bocrest Outrate EAL	DEPA	STATE RTMENT	OF NORTH CAR		ATION
038640 : =		SUBS	STRUCI	FURE	
8/8/2017		END E	BENT	No.	1
STV ENGINEERS, INC.		DE	ETAIL	_S	
100 900 West Trade St., Suite 715 Charlotte, NC 28202 NC License Number F-0991		REVIS	IONS		SHEET NO.
NT NOT CONSIDERED	NO. BY:	DATE:	NO. BY:	DATE:	S-13
NAL UNLESS ALL	1		3		TOTAL SHEETS

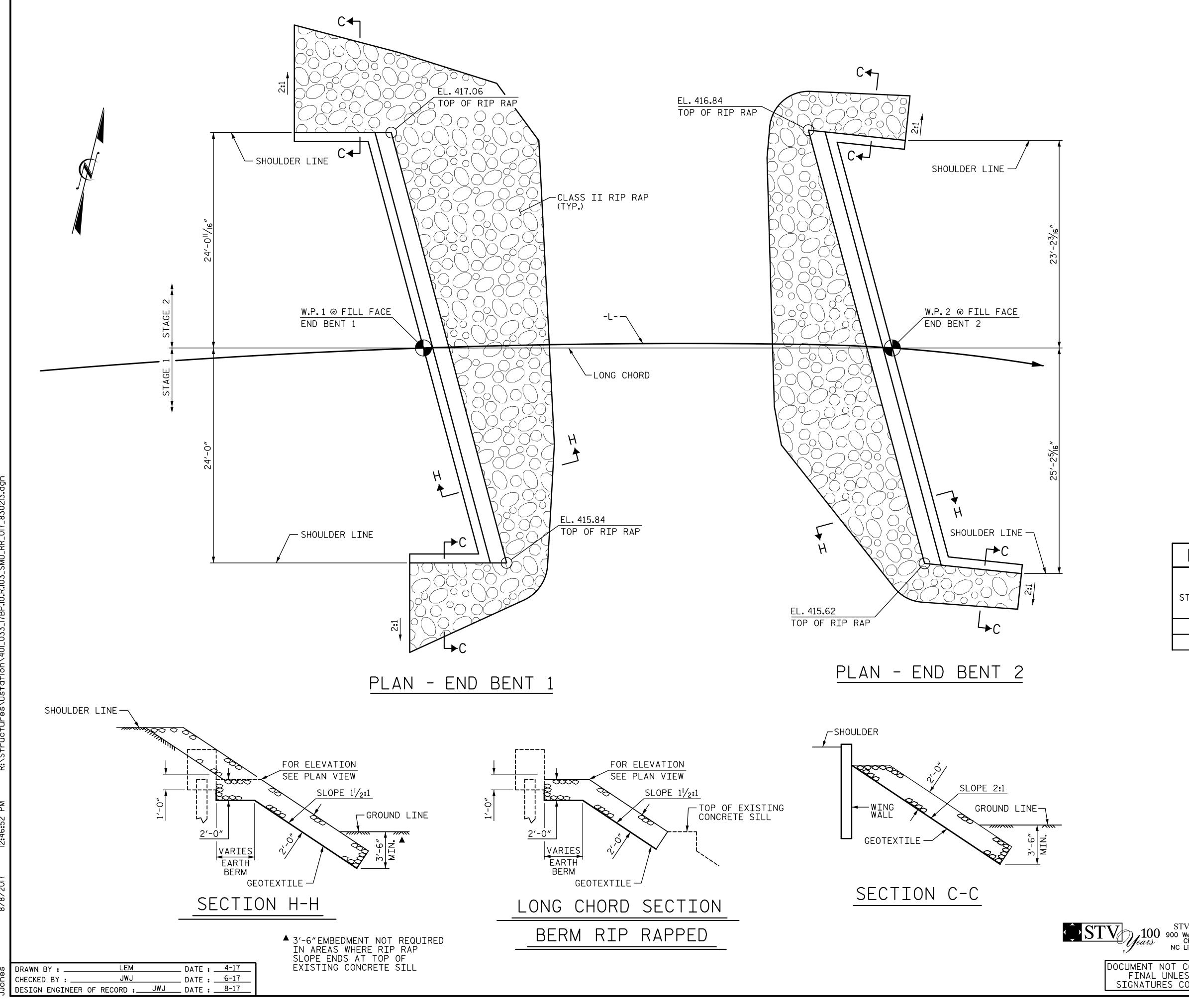






						1					
	BI	LL O	F MA	ATERIA	L		BI	LL O	<u>F M</u>	ATERIA	AL I
E	ND	BEN	T 2	(STAC	GE 1)	E	ND	BEN	T 2	(STA)	GE 2)
7	NO.	SIZE	TYPE	LENGTH	WEIGHT	BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT
	8	#9	1	26′-8″	725	B3	6	#4	STR	2'-5″	10
	14	#4	STR	25′-5″	238	B4	8	#9	1	25′-5″	691
1	7	#4	STR	2′-5″	11	B5	14	#4	STR	24'-3"	227
	14	#6	STR	1′-6″	32	D1	14	#6	STR	1'-6″	32
	10	#4	3	9′-7″	64	H1	10	#4	2	9'-0"	60
	10	#4	3	9'-4″	62	H2	10	#4	2	9'-3"	62
	8	#4	STR	3'-2″	17	K1	8	#4	STR	3'-2"	17
	31	#4	4	10′-5″	216	S1	31	#4	4	10′-5″	216
	31	#4	5	3'-2″	66	S2	31	#4	5	3'-2"	66
1	16	#4	6	6′-6″	69	S3	12	#4	6	6'-6″	52
	1	#4	4	10'-6″	7	S4	1	#4	4	10'-6″	7
	1	#4	5	3'-3"	2	S5	1	#4	5	3'-3"	2
	27	#4	STR	6'-2″	111	V1	26	#4	STR	6′-2″	107
ΕN	FORCI	NG STE	EL	10	620 LBS.	REIN	FORCI	NG STE	EL		1549 LBS.
SS	5 A C(DNCRETI	E BREA	AKDOWN		CLASS	A CO	DNCRETI	E BREA	KDOWN	
IR		CAP,LOWER PART 12.1 C.Y. POUR #1 CAP,LOWER PART 11.9 C. OF WINGS & COLLARS OF WINGS & COLLARS						11.9 C.Y.			
IR		PPER P INGS	ART O	١F	1.1 C.Y.	POUR		PPER P INGS	ART O	F	1.0 C.Y.
Al	L CLAS	SS A C	ONCRE	TE	13.2 C.Y.	TOTAL	L CLAS	SS A C	ONCRET	ſE	12.9 C.Y.

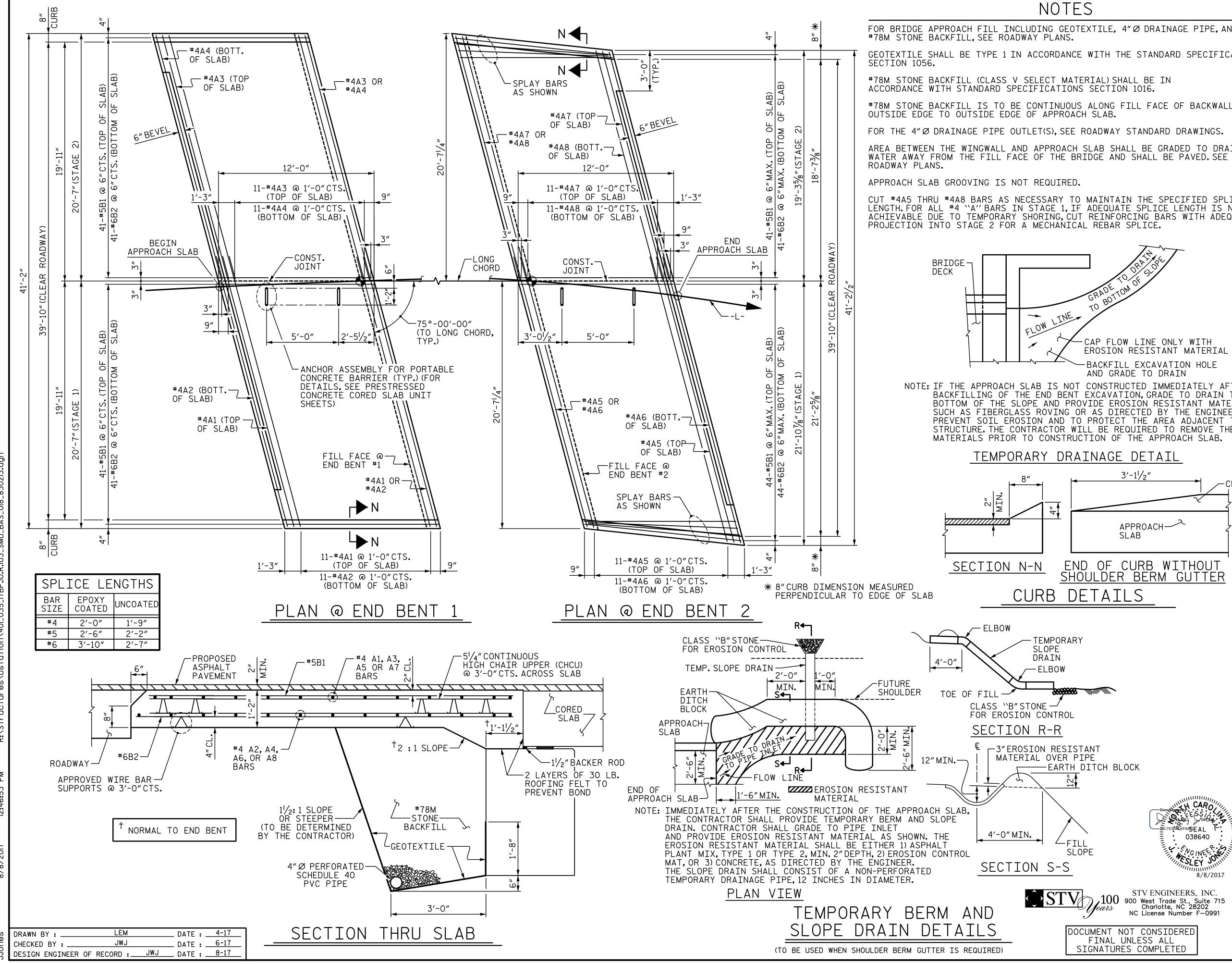
	PROJECT NO. 178P.10.R.103	_
	STANLY COUNTY	Y
	STATION: 14+65.00 -L-	_
	SHEET 3 OF 3	
Docusing to be CARO	STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH	
	SUBSTRUCTURE	
8/8/2017	END BENT No.2	
STV ENGINEERS, INC. 100 900 West Trade St., Suite 715 Charlotte, NC 28202	DETAILS	
NC License Number F-0991	REVISIONS SHEET	NO.
MENT NOT CONSIDERED FINAL UNLESS ALL	NO. BY: DATE: NO. BY: DATE: S-16 1 3 TOTAL SHEETS	
GNATURES COMPLETED	1 3 10 10 10 10 10 10 10 10 10 10	S



STATION: <u>14+65.00 -L-</u> STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH STV ENGINEERS, INC. West Trade St. Suite 715 Charlotte, NC 28202 License Number F-0991 CONSIDERED ESS ALL COMPLETED STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH RIP RAP DETAILS SHEET NO. S-17 TOTAL SHEET NO. SHEET NO. SH		PROJEC	CT NO. STAN			.03 UNTY
DEPARTMENT OF TRANSPORTATION RALEIGH DEPARTMENT OF TRANSPORTATION RALEIGH DEPARTMENT OF TRANSPORTATION RALEIGH RIP RAP DETAILS TV ENGINEERS, INC. West Trade St., Suite 715 Charlotte, NC 28202 C License Number F-0991 REVISIONS SHEET NO. S-17 1 SHEET NO. SHEET NO. SHEET SHEETS		STATI	ON:	14+65	.00 -L	
8/8/2017 STV ENGINEERS, INC. West Trade St., Suite 715 Charlotte, NC 28202 C License Number F-0991 REVISIONS SHEET NO. SHEET NO. SHEET NO. STOTAL SHEET NO. STOTAL SHEET NO. SHEET NO. STOTAL	Docusion do: CARO Docusion do: EESS Over 2 BEC7ESERD4F84SEAL 038640	DEPA		OF TRA		TION
West Trade St., Suite 715 Charlotte, NC 28202 C License Number F-0991 REVISIONS SHEET NO. CONSIDERED ESS ALL COMPLETED NO. BY: DATE: NO. BY: DATE: S-17 1 3 TOTAL SHEETS		R	IP R.	AP DI	ETAIL	S
C License Number F-0991 REVISIONS SHEET NO. CONSIDERED NO. BY: DATE: NO. BY: DATE: SHEET NO. ESS ALL 1 3 TOTAL COMPLETED SHEETS	West Trade St., Suite 715 Charlotte, NC 28202					
ESS ALL COMPLETED 1 TOTAL SHEETS	C License Number F-0991		REVIS	SIONS		
			DATE:		DATE:	

ESTIMATED QUANTITIES					
BRIDGE @ STA.14+65.00 -L-	RIP RAP CLASS II (2'-0" THICK)	GEOTEXTILE FOR DRAINAGE			
	TONS	SQUARE YARDS			
END BENT 1	120	130			
END BENT 2	95	105			

ESTIMATED QUANTITIES				
BRIDGE @ STA.14+65.00 -L-	RIP RAP CLASS II (2'-0" THICK)	GEOTEXTILE FOR DRAINAGE		
	TONS	SQUARE YARDS		
END BENT 1	120	130		
END BENT 2	95	105		



PM R:\Structures\Ustation\401_035_17BP.10.R.103_SMU_BAS_018_83C

+

	-					
ES					ATERIAL	
DTEXTILE, 4″Ø DRAINAGE PIPE, AND		PR(SLA TAGE	ABAT E 1)	EB 1
	BAR	NO.	SIZE	TYPE		WEIGHT
CE WITH THE STANDARD SPECIFICATIONS	* A1 A2	13 13	#4 #4	STR STR	23'-3" 23'-0"	202 200
TERIAL) SHALL BE IN	↓ D1	<u></u> 1	#5	STD	11/_1//	474
NS SECTION 1016. JS ALONG FILL FACE OF BACKWALL FROM	* B1 B2	41 41	#5 #6	STR STR	<u>11'-1"</u> <u>11'-7"</u>	474 713
CH SLAB.	REIN	FORCI	NG STE	EL	LBS.	913
ROADWAY STANDARD DRAWINGS.	•		DATED CING S	TEEL	LBS.	676
H SLAB SHALL BE GRADED TO DRAIN THE BRIDGE AND SHALL BE PAVED.SEE		~ ^ ^		-+	0. V	10.1
).			<u>concre</u> DACH		<u>c.y.</u> B AT I	12.1 EB 1
O MAINTAIN THE SPECIFIED SPLICE	,			TAGE		
IF ADEQUATE SPLICE LENGTH IS NOT JT REINFORCING BARS WITH ADEQUATE	BAR	NO.	SIZE	TYPE		WEIGHT
CAL REBAR SPLICE.	* A3 A4	13 13	#4 #4	STR STR	21'-0" 21'-0"	182 182
			#5	CTD	11/ 1//	47.4
	* B1 B2	41 41	#5 #6	STR STR	<u>11'-1"</u> 11'-7"	474 713
	DETN					
GRADETION	* EPC	XY CO	NG STE DATED		LBS.	895
GRADE TO DRATION OF SLOPE	REI	NFOR(CING S	TEEL	LBS.	656
	CLASS	S AA	CONCRE	ETE	C.Y.	12.1
- CAP FLOW LINE ONLY WITH EROSION RESISTANT MATERIAL	AF	PRC)ACH	SLA		EB 2
- BACKFILL EXCAVATION HOLE	BAR	NO.	(S SIZE	TAGE type	1)	WEIGHT
AND GRADE TO DRAIN T CONSTRUCTED IMMEDIATELY AFTER THE	* A5	13	#4	STR	24'-7"	213
T EXCAVATION, GRADE TO DRAIN TO THE	A6	13	#4	STR	24'-4"	211
OR AS DIRECTED BY THE ENGINEER TO	₩ B1	44	#5	STR	11'-1"	509
O PROTECT THE AREA ADJACENT TO THE WILL BE REQUIRED TO REMOVE THESE JCTION OF THE APPROACH SLAB.	B2	44	#6	STR	11'-7″	766
			NG STE	EL	LBS.	977
AINAGE DETAIL			DATED CING S	TEEL	LBS.	722
			CONCRE		<u> </u>	
			DACH		<u>с. ү.</u> В АТ Б	12.4 EB 2
	, , ,	1 1 1 1		TAGE		_0 _
APPROACH	BAR	NO.	SIZE	TYPE		WEIGHT
	* A7 A8	13 13	#4 #4	STR STR	20'-10" 20'-10"	181 181
ND OF CURB WITHOUT			#5			
IOULDER BERM GUTTER	* B1 B2	41 41	#5 #6	STR STR	11'-1" 11'-7"	474 713
DETAILS	DETNI		NG STE		LBS.	894
	* EPC	XY CO	DATED			
۲Y	REI	.NFOR(CING S	IEEL	LBS.	655
	CLASS	S AA	CONCRE	ETE	C. Y.	11.8
DD		Γ ΝΙ	า	17BF	P.10.R.1	03
	UJEU					
		51A	NLY		CO	UNTY
STANT PIPE ST	ATIO	N:	14	+65.	.00 -L-	-
I DITCH BLOCK						
<u>*</u>						
Docusing dubies contraction	DEPAR		TATE OF N		NSPORTA	TION
Crest Darba See AL F						

	STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH					
BRIDGE APPROACH SLAB FOR PRESTRESSED CONCRETE CORED SLAB UNIT (SUB-REGIONAL TIER) 75° SKEW						
REVISIONS				SHEET NO.		
N0.	BY:	DATE:	N0.	BY:	DATE:	S-18
1			ଙ୍ଚ			TOTAL SHEETS
2			ক্ষ			18

DESIGN DATA:

SPECIFICATIONS	A.A.S.H.T.O. (CURRENT)
SI LOTI TOATIONS	
LIVE LOAD	SEE PLANS
IMPACT ALLOWANCE	SEE A.A.S.H.T.O.
STRESS IN EXTREME FIBER OF	
STRUCTURAL STEEL - AASHTO M270 GRADE 36 -	20,000 LBS.PER SQ.IN.
- AASHTO M270 GRADE 50W -	27,000 LBS.PER SQ.IN.
- AASHTO M270 GRADE 50 -	27,000 LBS.PER SQ.IN.
REINFORCING STEEL IN TENSION	
GRADE 60	24,000 LBS.PER SQ.IN.
CONCRETE IN COMPRESSION	1,200 LBS.PER SQ.IN.
CONCRETE IN SHEAR	SEE A.A.S.H.T.O.
STRUCTURAL TIMBER - TREATED OR	
UNTREATED - EXTREME FIBER STRESS	1,800 LBS.PER SQ.IN.
COMPRESSION PERPENDICULAR TO GRAIN	
OF TIMBER	375 LBS.PER SQ.IN.
EQUIVALENT FLUID PRESSURE OF EARTH	30 LBS.PER CU.FT.
	(MINIMUM)

MATERIAL AND WORKMANSHIP:

EXCEPT AS MAY OTHERWISE BE SPECIFIED ON PLANS OR IN THE SPECIAL PROVISIONS, ALL MATERIAL AND WORKMANSHIP SHALL BE IN ACCORDANCE WITH THE 2012 "STANDARD SPECIFICATIONS FOR ROADS AND STRUCTURES" OF THE N. C. DEPARTMENT OF TRANSPORTATION.

STEEL SHEET PILING FOR PERMANENT OR TEMPORARY APPLICATIONS SHALL BE HOT ROLLED.

CONCRETE:

UNLESS OTHERWISE REQUIRED ON PLANS, CLASS A CONCRETE SHALL BE USED FOR ALL PORTIONS OF ALL STRUCTURES WITH THE EXCEPTION THAT: CLASS AA CONCRETE SHALL BE USED IN BRIDGE SUPERSTRUCTURES, ABUTMENT BACKWALLS, AND APPROACH SLABS; AND CLASS B CONCRETE SHALL BE USED FOR SLOPE PROTECTION AND RIP RAP.

CONCRETE CHAMFERS:

UNLESS OTHERWISE NOTED ON THE PLANS, ALL EXPOSED CORNERS ON STRUCTURES SHALL BE CHAMFERED 3/4" WITH THE FOLLOWING EXCEPTIONS: TOP CORNERS OF CURBS MAY BE ROUNDED TO 1-1/2" RADIUS WHICH IS BUILT INTO CURB FORMS: CORNERS OF TRANSVERSE FLOOR EXPANSION JOINTS SHALL BE ROUNDED WITH A 1/4"FINISHING TOOL UNLESS OTHERWISE REQUIRED ON PLANS: AND CORNERS OF EXPANSION JOINTS IN THE ROADWAY FACES AND TOPS OF CURBS AND SIDEWALKS SHALL BE ROUNDED TO A 1/4"RADIUS WITH A FINISHING STONE OR TOOL UNLESS OTHERWISE REQUIRED ON PLANS.

DOWELS:

DOWELS WHEN INDICATED ON PLANS AS FOR CULVERT EXTENSIONS, SHALL BE EMBEDDED AT LEAST 12" INTO THE OLD CONCRETE AND GROUTED INTO PLACE WITH 1:2 CEMENT MORTAR.

STANDARD NOTES

ALLOWANCE FOR DEAD LOAD DEFLECTION, SETTLEMENT, ETC. IN CASTING SUPERSTRUCTURES:

BRIDGES SHALL BE BUILT ON THE GRADE OR VERTICAL CURVE SHOWN ON PLANS. SLABS, CURBS AND PARAPETS SHALL CONFORM TO THE GRADE OR CURVE. ALL DIMENSIONS WHICH ARE GIVEN IN SECTION AND ARE AFFECTED BY DEAD LOAD DEFLECTIONS ARE DIMENSIONS AT CENTER LINE OF BEARING UNLESS OTHERWISE NOTED ON PLANS. IN SETTING FORMS FOR STEEL BEAM BRIDGES AND PRESTRESSED CONCRETE GIRDER BRIDGES, ADJUSTMENTS SHALL BE MADE DUE TO THE DEAD LOAD DEFLECTIONS FOR THE ELEVATIONS SHOWN. WHERE BLOCKS ARE SHOWN OVER BEAMS FOR BUILDING UP TO THE SLAB, THE VERTICAL DIMENSIONS OF THE BLOCKS SHALL BE ADJUSTED BETWEEN BEARINGS TO COMPENSATE FOR DEAD LOAD DEFLECTIONS, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER. WHERE BOTTOM OF SLAB IS IN LINE WITH BOTTOM OF TOP FLANGES, DEPTH OF SLAB BETWEEN BEARINGS SHALL BE ADJUSTED TO COMPENSATE FOR DEAD LOAD DEFLECTION, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER.

IN SETTING FALSEWORK AND FORMS FOR REINFORCED CONCRETE SPANS, AN ALLOWANCE SHALL BE MADE FOR DEAD LOAD DEFLECTIONS, SETTLEMENT OF FALSEWORK, AND PERMANENT CAMBER WHICH SHALL BE PROVIDED FOR IN ADDITION TO THE ELEVATIONS SHOWN. AFTER REMOVAL OF THE FALSEWORK, THE FINISHED STRUCTURES SHALL CONFORM TO THE PROFILE AND ELEVATIONS SHOWN ON THE PLANS AND CONSTRUCTION ELEVATIONS FURNISHED BY THE ENGINEER.

DETAILED DRAWINGS FOR FALSEWORK OR FORMS FOR BRIDGE SUPERSTRUCTURE AND ANY STRUCTURE OR PARTS OF A STRUCTURE AS NOTED ON THE PLANS SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL BEFORE CONSTRUCTION OF THE FALSEWORK OR FORMS IS STARTED.

REINFORCING STEEL:

ALL REINFORCING STEEL SHALL BE DEFORMED. DIMENSIONS RELATIVE TO PLACEMENT OF REINFORCING ARE TO CENTERS OF BARS UNLESS OTHERWISE INDICATED IN THE PLANS. DIMENSIONS ON BAR DETAILS ARE TO CENTERS OF BARS OR ARE OUT TO OUT AS INDICATED ON PLANS.

WIRE BAR SUPPORTS SHALL BE PROVIDED FOR REINFORCING STEEL WHERE INDICATED ON THE PLANS. WHEN BAR SUPPORT PIECES ARE PLACED IN CONTINUOUS LINES, THEY SHALL BE SO PLACED THAT THE ENDS OF THE SUPPORTING WIRES SHALL BE LAPPED TO LOCK LEGS ON ADJOINING PIECES.

STRUCTURAL STEEL:

AT THE CONTRACTOR'S OPTION, HE MAY SUBSTITUTE 7/8" Ø SHEAR STUDS FOR THE ¾″Ø STUDS SPECIFIED ON THE PLANS. THIS SUBSTITUTION SHALL BE MADE AT THE RATE OF 3 - 7/8"Ø STUDS FOR 4 - 3/4"Ø STUDS, AND STUD SPACING CHANGES SHALL BE MADE AS NECESSARY TO PROVIDE THE SAME EQUIVALENT NUMBER OF 7/8"Ø STUDS ALONG THE BEAM AS SHOWN FOR 3/4" Ø STUDS BASED ON THE RATIO OF 3 - 7/8" Ø STUDS FOR 4 - 3/4" Ø STUDS. STUDS OF THE LENGTH SPECIFIED ON THE PLANS MUST BE PROVIDED. THE MAXIMUM SPACING SHALL BE 2'-O".

EXCEPT AT THE INTERIOR SUPPORTS OF CONTINUOUS BEAMS WHERE THE COVER PLATE IS IN CONTACT WITH BEARING PLATE, THE CONTRACTOR MAY, AT HIS OPTION, SUBSTITUTE FOR THE COVER PLATES DESIGNATED ON THE PLANS COVER PLATES OF THE EQUIVALENT AREA PROVIDED THESE PLATES ARE AT LEAST 5/16" IN THICKNESS AND DO NOT EXCEED A WIDTH EQUAL TO THE FLANGE WIDTH LESS 2"OR A THICKNESS EQUAL TO 2 TIMES THE FLANGE THICKNESS. THE SIZE OF FILLET WELDS SHALL CONFORM TO THE REQUIREMENTS OF THE CURRENT ANSI/AASHTO/AWS "BRIDGE WELDING CODE". ELECTROSLAG WELDING WILL NOT BE PERMITTED. WITH THE SOLE EXCEPTION OF EDGES AT SURFACES WHICH BEAR ON OTHER

SURFACES.ALL SHARP EDGES AND ENDS OF SHAPES AND PLATES SHALL BE SLIGHTLY ROUNDED BY SUITABLE MEANS TO A RADIUS OF APPROXIMATELY 1/16 INCH OR EQUIVALENT FLAT SURFACE AT A SUITABLE ANGLE PRIOR TO PAINTING, GALVANIZING, OR METALLIZING.

METAL STANDARDS AND FACES OF THE CONCRETE END POSTS FOR THE METAL RAIL SHALL BE SET NORMAL TO THE GRADE OF THE CURB, UNLESS OTHERWISE SHOWN ON PLANS. THE METAL RAIL AND TOPS OF CONCRETE POSTS USED WITH THE ALUMINUM RAIL SHALL BE BUILT PARALLEL TO THE GRADE OF THE CURB. METAL HANDRAILS SHALL BE IN ACCORDANCE WITH THE PLANS. RAILS SHALL BE AS MANUFACTURED FOR BRIDGE RAILING. CASTINGS SHALL BE OF A UNIFORM APPEARANCE. FINS AND OTHER DEFORMATIONS RESULTING FROM CASTING OR OTHERWISE SHALL BE REMOVED IN A MANNER SO THAT A UNIFORM COLORING OF THE COMPLETED CASTING SHALL BE OBTAINED. CASTINGS WITH DISCOLORATIONS OR OF NON-UNIFORM COLORING WILL NOT BE ACCEPTED. CERTIFIED MILL REPORTS ARE REQUIRED FOR METAL RAILS AND POSTS.

SPECIAL NOTES:

GENERALLY. IN CASE OF DISCREPANCY. THIS STANDARD SHEET OF NOTES SHALL GOVERN OVER THE SPECIFICATIONS, BUT THE REMAINDER OF THE PLANS SHALL GOVERN OVER NOTES HEREON, AND SPECIAL PROVISIONS SHALL GOVERN OVER ALL. SEE SPECIFICATIONS ARTICLE 105-4.

HANDRAILS AND POSTS:

